PROPOSED MAINTENANCE FACILITY - SHS-SYS.INV

LRHSD PROJECT NO. REF21-43-8319

SHAWNEE HIGH SCHOOL PROPOSED MAINTENANCE BUILDING
580 TABERNACLE ROAD
MEDFORD, NJ 08055

LENAPE REGIONAL HIGH SCHOOL SCHOOL DISTRICT

PREPARED FOR THE

LENAPE REGIONAL HIGH SCHOOL DISTRICT BOARD OF EDUCATION

SAMUEL B. GREEN PRESIDENT
JOANNE E. HARMON VICE PRESIDENT
MATTHEW D. WEBB SUPERINTENDENT

CIVIL ENGINEER
ARTHEON
CONSULTING AND MUNICIPAL ENGINEERS
344 NORTH ROUTE 73
BERLIN, NJ 08009

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ARCHITECT
THE DESIGN COLLABORATIVE
ARCHITECTS AND PLANNERS, P.A.
1940 ROUTE 9 NORTH
CAPE MAY COURT HOUSE, NJ 08210

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MPE ENGINEER
CONCORD ENGINEERING
CONSULTING ENGINEERS
520 SOUTH BURNT MILL ROAD
VOORHEES, NJ 08043

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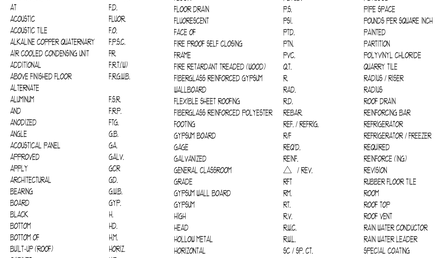
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REF21-43-8319 SHAWNEE HIGH SCHOOL PROPOSED MAINTENANCE BUILDING - BID RELEASE OCTOBER, 2026

ABBREVIATION LIST

AB.	ANCHOR BOLTS	FL / FLR.	FLOOR	PL	PLATE
@	AT	FD.	FLOOR DRAIN	PLYWD.	PLYWOOD
ACOUS.	ACOUSTIC	FLUOR.	FLUORESCENT	PS.	PIPE SPACE
ACT(/ATJ)	ACOUSTIC TILE	F.O.	FACE OF	PTD.	PAINTED
ACQ.	ALKALINE COPPER QUATERNARY	FPSC.	FIRE PROOF SELF CLOSING	PTN.	PARTITION
ACCU.	AIR COOLED CONDENSING UNIT	FR.	FRAME	PVC.	POLYVINYL CALORIDE
ADDL.	ADDITIONAL	FRT(W)	FIRE RETARDANT TREADED (WOOD)	QT.	QUARRY TILE
AFF.	ABOVE FINISHED FLOOR	FRP&GB.	FIBERGLASS REINFORCED GYPSUM	R.	RADIUS / RISER
ALT.	ALTERNATE		WALLBOARD	RAD.	RADIUS
ALUM / AL.	ALUMINUM	FSR.	FLEXIBLE SHEET ROOFING	RD.	ROOF DRAIN
&	AND	FRP.	FIBERGLASS REINFORCED POLYESTER	REBAR.	REINFORCING BAR
ANOD.	ANODIZED	FTG.	FOOTING	REF. / REFRIG.	REFRIGERATOR
∠ / ∠S	ANGLE	GB.	GYPSUM BOARD	RIF.	REFRIGERATOR / FREEZER
APP.	ACOUSTICAL PANEL	GA.	GAUGE	REQ'D.	REQUIRED
APP'D	APPROVED	GALV.	GALVANIZED	REINF.	REINFORCE (ING)
APPL.	APPLY	GCR	GENERAL CLASSROOM	△ / REV.	REVISION
ARCH.	ARCHITECTURAL	GD.	GRADE	RBT	RUBBER FLOOR TILE
BRG.	BEARING	GWB.	GYPSUM WALL BOARD	RM.	ROOM
BD.	BOARD	GYP.	GYPSUM	RT.	ROOF TOP
BLK.	BLACK	H.	HIGH	R.V.	ROOF VENT
BOT. / B.	BOTTOM	HD.	HEAD	RWC.	RAIN WATER CONDUCTOR
B.O.	BOTTOM OF	HM.	HOLLOW METAL	RWL.	RAIN WATER LEADER
BUR(R)	BUILT-UP (ROOF)	HORIZ.	HORIZONTAL	SC / SP. CT.	SPECIAL COATING
CAB.	CABINET	HP.	HIGH POINT	SF	SQUARE FOOT
CB.	CHALKBOARD	HPS.	HIGH PRESSURE SODIUM	SHT.	SHEET
CCA.	COPPER CROMATED ARSENIC	HR.	HANDRAIL	SIM.	SIMILAR
CER.	CERAMIC	HT.	HEIGHT	SPEC (S.)	SPECIFICATION (S)
C.J.	CONTROL JOINT	H.V.A.C.	HEATING, VENTILATION, &	SQ.	SQUARE
C.T.	CERAMIC TILE		AIR CONDITIONING	STD.	STANDARD
CH / C.	CHANNEL	IARR	INTERLOCKING ATHLETIC RUBBER	STG.	STORAGE
CL / ℄	CENTER LINE		FLOORING	STL / ST.	STEEL
CL.	CLOSET	ID.	INSIDE DIMENSION	SS / ST. ST.	STAINLESS STEEL
CLG.	CEILING	INSUL.	INSULATION	STRUCT.	STRUCTURAL
COL.	COLUMN	JB.	JAMB	SUSP'D.	SUSPENDED
CMU	CONCRETE MASONRY UNIT	JT.	JOINT	SW.	SHORT WAY
CNTR.	COUNTER	KIT.	KITCHEN	T.	TREAD / THICKNESS / TOP
CONC.	CONCRETE	LG.	LONG	T.B.	TACK BOARD
CONSTR.	CONSTRUCTION	LAM.	LAMINATED	T. & B.	TOP AND BOTTOM
CONTR.	CONTRACTOR	LAB.	LABEL	TEL.	TELEPHONE
CORR.	CORRIDOR	LAV.	LAVATORY	THK.	THICK
CONT.	CONTINUOS	LL.H.	LONG LEG HORIZONTAL	TLT.	TOILET
D.	DEEP	LL.V.	LONG LEG VERTICAL	T.O.	TOP OF
DET.	DETAIL	L.P.	LOW POINT	T.O.S.	TOP OF STEEL / TOP OF SLAB
⌀/DIA.	DIAMETER	LWC.	LIGHTWEIGHT CONCRETE	T.S.	TRANSITION STRIP / CARPET TO VINYL
DIST.	DISTRIBUTION (TOR)	MACH.	MACHINE	T.O.W.	TOP OF WALL
DBL.	DOUBLE	MAS.	MASONRY	TYP.	TYPICAL
DMB.	DRY MARKER BOARD	MAX.	MAXIMUM	U.C.	UTILITY CLOSET
DR.	DOOR	M.B.&R.	MODIFIED BITUMEN SHEET ROOFING	U.L.	UNDERWRITER'S LABORATORIES
D.S.	DOWN SPOUT	MDO.	MARINE DENSITY OVERLAY	U.N.O.	UNLESS NOTED OTHERWISE
DWG(S).	DRAWING (S)	MECH.	MECHANICAL	U.S.	UNDER SIDE
EA.	EACH	MFGR.	MANUFACTURER	USG.	UNITED STATES GYPSUM
EFF.(C)	EFFICIENCY	MFG'D.	MANUFACTURED	U.V.	UNIT VENTILATOR
E.J.	EXPANSION JOINT	MFS.	MONOLITHIC EPOXY	VCT.	VINYL COMPOSITION TILE
EL. / ELEV.	ELEVATION		FLOOR SYSTEM	VERT.	VERTICAL
E.P.	ELECTRIC PANEL	MIN.	MINIMUM	V.H.I.F.	VERY HIGH IMPACT FIBERGLASS
EPDM.	ETHYLENE PROPYLENE DIENE-	M.O.	MASONRY OPENING		REINFORCED
	MONOMER	MT.	MARBLE THRESHOLD	V.I.F.	VERIFY IN FIELD
EPS.	EXTRUDED POLYSTYRENE	MTD.	MOUNTED	V.TR.	VENT THRU ROOF
EQ.	EQUAL	MTL.	METAL	V.V.I.M.	VERIFY WITH MANUFACTURER
EQUIV.	EQUIVALENT	MW.	MICROWAVE	W.	WIDTH
EQUIP.	EQUIPMENT	NC.	NO CEILING	WD.	WOOD
E.S.	EACH SIDE	N.I.C.	NOT IN CONTRACT	W/	WITH
E.W.C.	ELECTRIC WATER COOLER	NO.	NUMBER	W.R.	WATER RESISTANT
EXG. / EXIST.	EXISTING	N.T.S.	NOT TO SCALE	W.W.F.	WELDED WIRE FABRIC
EXP.	EXPANSION	O.C.	ON CENTER	W.P.	WATER PROOF
EXT.	EXTERIOR	OPER.	OPERATOR / TION	W.C.	WATER CLOSET
F.A.	FIRE ALARM	O.D.	OUTSIDE DIAMETER	W.H.	WALL HYDRANT
F.E.	FIRE EXTINGUISHER	PART.	PARTITION	Y.D.	YARD DRAIN
FF.	FINISH FLOOR	P.C.	PRE-CAST		
FIN.	FINISH				

LOCATION MAP



PROJECT SCOPE

THE SCOPE OF THIS PROJECT INCLUDES ARCHITECTURAL AND LIMITED STRUCTURAL RENOVATIONS AND ALTERATIONS AS REQUIRED TO RESTORE THE EXISTING SIDING AND ROOFING AT THE DISTRICT TRANSPORTATION BUILDING.

CODE INFORMATION

USE GROUP:	B-1 - MODERATE HAZARD STORAGE
CONSTRUCTION TYPE:	5B
FIRE GRADING:	2 HOUR FLAME SPREAD, INTERIOR FINISH (ROOMS 4 ENCLOSED SPACES, IBC TABLE 803.9) CLASS C. INTERIOR FLOOR FINISH CLASS II, IBC 804.4)
USE OF BUILDING:	STORAGE / MAINTENANCE. NO CHANGE IN USE IS PROPOSED UNDER THE SCOPE OF THIS APPLICATION.
BUILDING HEIGHT:	1 STORY, 24'-0" NOMINAL
ZONING REQUIREMENTS:	APPROVED EDUCATIONAL ACCESSORY USE

THE FOLLOWING CODES ARE APPLICABLE TO THIS PROJECT.

A. NEW JERSEY UNIFORM CONSTRUCTION CODE, 5:23, AS AMENDED, NEW JERSEY DEPARTMENT OF COMMUNITY AFFAIRS, SHALL GOVERN THE WORK OF THIS PROJECT. EXCEPT AS NOTED HEREIN. ALSO SUBCODES AS APPLICABLE TO THE WORK.
B. NEW JERSEY BARRIER-FREE SUBCODE, 5:23-7, AS AMENDED.
C. INTERNATIONAL BUILDING CODE (IBC) ED., 2021
D. INTERNATIONAL MECHANICAL CODE, 2021, AS AMENDED.
E. INTERNATIONAL ENERGY CONSERVATION CODE, (2021 (RESIDENTIAL) AND ASHRAE 90. 1-2019 (COMMERCIAL) INTERNATIONAL CODE COUNCIL, INC. (ICC).
F. NATIONAL STANDARD PLUMBING CODE, 2021, AS AMENDED.
G. ELECTRICAL SUBCODE, NATIONAL ELECTRICAL CODE / NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) 70 / 2020, AS AMENDED.
H. REHABILITATION SUBCODE - NJAC 5:23-6, AND SPECIFICALLY 5:23-6.4
I. OSHA REGULATIONS, AS APPLICABLE.
J. ADA (American with Disabilities Act) AS APPLICABLE.
K. (NJAC) NEW JERSEY ADMINISTRATIVE CODE, TITLE 6A CHAPTER 32, AS APPLICABLE.
L. ICC / ANSI A117.1, 2017
M. INTERNATIONAL FUEL GAS CODE, 2021, AS APPLICABLE
N. NFPA 13, 2020.D
O. REFERENCE (AMERICAN SOCIETY OF CIVIL ENGINEERS) CURRENT EDITION.
P. AJMA / UCPA / CSA 101 / I.S2 / AAMD-09 AND AAMD-08.
Q. ASTM E1886-05 / ASTM E1996-05 WIND ZONE AND MISSILE TESTING.

GENERAL NOTES:

1. SCHEDULE: SUBMIT SCHEDULE INDICATING PROPOSED METHODS AND SEQUENCE OF OPERATIONS FOR SELECTIVE DEMOLITION WORK TO ARCHITECT FOR REVIEW PRIOR TO COMMENCEMENT OF WORK. INCLUDE COORDINATION FOR SHUTOFF, CAPPING, AND CONTINUATION OF UTILITY SERVICES AS REQUIRED. TOGETHER W DETAILS FOR DUST AND NOISE CONTROL. PROVIDE DETAILED SEQUENCE OF DEMOLITION AND REMOVAL WORK TO ENSURE OWNER'S ONSITE OPERATIONS ARE NOT UNINTERRUPTED. COORDINATE WITH OWNER'S CONTINUING OCCUPATION OF PORTIONS OF EXISTING BUILDING.
2. STORAGE OR SALE OF REMOVED ITEMS ON SITE WILL NOT BE PERMITTED.
3. PROTECTIONS: PROVIDE TEMPORARY BARRICADES AND OTHER FORMS OF PROTECTION AS REQUIRED TO PROTECT OWNER'S PERSONNEL AND GENERAL PUBLIC FROM INJURY DUE TO DEMOLITION WORK.
4. PROVIDE PROTECTIVE MEASURES AS REQUIRED TO PROVIDE FREE AND SAFE PASSAGE OF OWNER PERSONNEL TO AND FROM OCCUPIED PORTIONS OF BUILDING.
5. PROTECT FROM DAMAGE EXISTING FINISH WORK THAT IS TO REMAIN IN PLACE AND BECOMES EXPOSED.
6. CONSTRUCT TEMPORARY INSULATED SOLID DUSTPROOF PARTITIONS WHERE REQUIRED TO SEPARATE AREAS WHERE NOISY OR EXTENSIVE DIRT OR DUST OPERATIONS ARE PERFORMED. EQUIP PARTITIONS WITH DUSTPROOF DOORS AND SECURITY LOCKS IF REQUIRED. DAMAGES: PROMPTLY REPAIR DAMAGES CAUSED TO ADJACENT FACILITIES BY DEMOLITION WORK AT NO COST TO OWNER.
7. UTILITY SERVICES: MAINTAIN EXISTING UTILITIES INDICATED TO REMAIN, KEEP IN SERVICE, AND PROTECT AGAINST DAMAGE DURING DEMOLITION OPERATIONS. DO NOT INTERRUPT EXISTING UTILITIES SERVING OCCUPIED OR USED FACILITIES EXCEPT WHEN AUTHORIZED IN WRITING BY AUTHORITIES HAVING JURISDICTION. PROVIDE TEMPORARY SERVICES DURING INTERRUPTIONS TO EXISTING UTILITIES, AS ACCEPTABLE TO GOVERNING AUTHORITIES.

SYMBOLS



GENERAL NOTES AND CONTRACT SPECIFICATIONS

THE FOLLOWING DEMOLITION NOTES SHALL APPLY TO ALL CONDITIONS, WHETHER SPECIFICALLY INDICATED OR NOT.

DIVISION 1: GENERAL NOTES AND REQUIREMENTS

1. ALL WORK INCLUDED IN THESE PLANS AND SPECIFICATIONS SHALL BE GOVERNED BY THE CONTRACT DOCUMENTS AS DEFINED BY THE GENERAL CONDITIONS OF THE CONTRACT FOR CONSTRUCTION A.I.A. A201, LATEST EDITION.
2. THE CONTRACTOR SHALL VISIT THE SITE AND FAMILIARIZE HIMSELF WITH ALL EXISTING CONDITIONS AND VERIFY THE LOCATION AND EXISTENCE OF ALL IMPROVEMENTS, BOTH ABOVE AND BELOW THE GROUND SURFACE PRIOR TO THE INITIATION OF ANY WORK. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING AS REQUIRED. THE CONTRACTOR SHALL AT ALL TIMES ADEQUATELY PROTECT THE EXISTING PROPERTY AND SITE IMPROVEMENTS AGAINST DAMAGE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE AS A RESULT OF CONTRACT OPERATIONS, AND SHALL REPLACE OR REPAIR DAMAGED AREAS TO THEIR ORIGINAL CONDITION.
3. ALL WORK SHALL BE DONE IN STRICT CONFORMANCE WITH ALL STATE AND LOCAL CODES AND ORDINANCES.
4. ALL WORK SHALL BE DONE IN A MANNER CONSISTENT WITH THE HIGHEST STANDARDS OF THE RESPECTIVE TRADES AND CONSISTENT WITH INDUSTRY STANDARDS.
5. REFER TO PLUMBING, HVAC AND ELECTRICAL DRAWINGS FOR INFORMATION REGARDING BUILDING MECHANICAL, PLUMBING AND ELECTRICAL IMPROVEMENTS.

GENERAL CONSTRUCTION

1. NOTES, TYPICAL DETAILS, AND SCHEDULES APPLY TO ALL WORK UNLESS NOTED OTHERWISE. TYPICAL DETAILS ARE TO BE USED FOR ALL CONDITIONS WHERE THE DETAIL IS APPLICABLE, WHETHER OR NOT NOTED ON PLAN. TYPICAL DETAILS MAY BE SLIGHTLY ALTERED IF REQUIRED DUE TO PROJECT CONDITIONS, ONLY WHEN SUBMITTED AND THE ARCHITECT'S APPROVAL IS OBTAINED PRIOR TO PERFORMING THE WORK.
2. IF THERE IS A DISCREPANCY BETWEEN DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO PERFORMING THE WORK.
3. IF DIFFERENCES OCCUR WITHIN OR BETWEEN DRAWINGS AND SPECIFICATIONS REGARDING MATERIALS, STRENGTHS OR QUANTITIES, THE BETTER MATERIAL, HIGHER STRENGTH, AND GREATER QUANTITY INDICATED, SPECIFIED OR NOTED SHALL BE PROVIDED.
4. DO NOT SCALE DRAWINGS TO OBTAIN DIMENSIONAL INFORMATION.
5. THESE DRAWINGS DO NOT DEFINE SCOPE OF CONTRACTOR OR SUBCONTRACTOR CONTRACTS.
6. AT ALL TIMES, THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOBSITE INCLUDING MEANS AND METHODS OF CONSTRUCTION AND SAFETY OF PERSONS AND PROPERTY. THE ARCHITECT'S/ENGINEER'S PRESENCE OR REVIEW OF WORK AT THE JOBSITE IS FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT ONLY AND IS NOT EVER TO BE CONSTRUED AS A REVIEW OF MEANS AND METHODS OF CONSTRUCTION AND SAFETY METHODS.
7. COSTS OF INVESTIGATION AND/OR REDESIGN DUE TO CONTRACTOR ERRORS WILL BE AT THE CONTRACTOR'S EXPENSE.
8. ANY APPROVED CONTRACTOR REQUESTED CHANGES TO THESE DRAWINGS WILL BE DONE AT NO COST TO THE OWNER. APPROVAL OF CONTRACTOR REQUESTED CHANGES SHALL BE AT THE DISCRETION OF THE OWNER.
9. SIZE AND/OR LOCATION OF EXISTING STRUCTURES AND UTILITIES SHOWN ON THE DOCUMENTS ARE FOR THE CONTRACTOR'S CONVENIENCE ONLY. THE CONTRACTOR IS SOLELY RESPONSIBLE TO VERIFY BY FIELD MEASUREMENTS/INVESTIGATION THE SIZE AND/OR LOCATION OF ALL EXISTING STRUCTURES AND UTILITIES.

STAGING AND CONSTRUCTION SITE SAFETY

1. SAFETY PLAN: THE CONTRACTOR SHALL BE REQUIRED TO SUBMIT A PROJECT SAFETY PLAN ADDRESSING THE SAFETY REQUIREMENTS FOR THIS SPECIFIC PROJECT. THE SAFETY PLAN SHALL INCLUDE THE CONTRACTOR'S SCHEDULE AND SEQUENCE OF OPERATIONS. DEVIATIONS IN THE STAGING AND PHASING PLAN INCLUDED WITHIN THESE DOCUMENTS WILL BE CONSIDERED, BUT ARE SUBJECT TO THE APPROVAL OF THE ARCHITECT, OWNER AND AHJ. THE PROJECT SAFETY PLAN SHALL FURTHER INCLUDE THE CONTRACTOR'S PROCEDURES AND PROTOCOLS RELATIVE TO THE MITIGATION OF THE THREAT POSED BY COVID-19. THE CONTRACTOR SHALL DESIGNATE AN INDIVIDUAL TO ACT AS ITS SAFETY MANAGER. THE SAFETY MANAGER SHALL BE REQUIRED TO BE ON-SITE AT ALL TIMES THAT CONSTRUCTION IS IN PROGRESS.
2. MATERIAL STORAGE AND DUMPSTER PLACEMENT: MATERIAL LAYDOWN AND DUMPSTER LOCATIONS HAVE BEEN DESIGNATED ON THE PLANS. MATERIAL LAYDOWN AND DUMPSTER LOCATIONS ARE TO BE ENCLOSED WITHIN A LOCKED, 8'-0" HIGH CHAINLINK CONSTRUCTION FENCE. SECURING AND PROTECTION OF ALL MATERIALS STORED ON SITE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. BUILDING MATERIALS, TOOLS AND EQUIPMENT SHALL NOT BE STORED OR LEFT WITHIN THE PUBLIC RIGHT-OF-WAY AT ANY TIME, AND MUST BE SECURED.
3. DUMPSTERS: DUMPSTERS WILL NOT BE PERMITTED TO BE PLACED ADJACENT TO THE BUILDING. DEMOLITION DEBRIS IS TO BE COLLECTED AND TRANSPORTED TO THE LOCATION DESIGNATED ON THE PLANS. IN THE EVENT THAT THE CONTRACTOR ELECTS TO ACCESS THE ROOF, OR TRANSPORT CONSTRUCTION MATERIALS AND / OR DEMOLITION DEBRIS ACROSS THE ROOF, THE CONTRACTOR SHALL BE REQUIRED TO PROVIDE PROTECTION MATS ACROSS ALL TRAFFIC PATHS AND STAGING AREAS. ROOF LOADING SHALL BE DISTRIBUTED SO AS TO NOT ADVERSELY IMPACT THE EXISTING ROOF MEMBRANE, INSULATION, ROOF ACCESSORIES OR ROOF MOUNTED EQUIPMENT. ROOF SURFACES SHALL BE CLEANED DAILY.
4. ACCESS: DO NOT CLOSE, BLOCK OR OTHERWISE OBSTRUCT STREETS, AMBULANCE OR EMERGENCY VEHICLE ACCESS, DELIVERY ACCESS, WALKS OR OTHER OCCUPIED OR USED FACILITIES WITHOUT WRITTEN PERMISSION FROM AUTHORITIES HAVING JURISDICTION. PROVIDE ALTERNATE ROUTES AROUND CLOSED OR OBSTRUCTED TRAFFIC WAYS IF REQUIRED BY GOVERNING REGULATIONS.
5. TRAFFIC: CONDUCT CONSTRUCTION ACTIVITIES, SELECTIVE DEMOLITION OPERATIONS AND DEBRIS REMOVAL IN A MANNER TO ENSURE MINIMUM INTERFERENCE WITH ROADS, STREETS, WALKS, AND OTHER ADJACENT OCCUPIED OR USED FACILITIES. STREET OR SIDEWALK CLOSURES WILL NOT BE PERMITTED.
6. PROTECTIONS: PROVIDE PROTECTIVE MEASURES AS REQUIRED TO PROVIDE FREE AND SAFE PASSAGE. PROVIDE TEMPORARY BARRICADES, WORK AIDS AND OTHER FORMS OF PROTECTION AS REQUIRED TO PROTECT OWNER'S PERSONNEL AND THE GENERAL PUBLIC FROM INJURY DUE TO DEMOLITION OR NEW CONSTRUCTION WORK.
7. ENVIRONMENTAL CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION. DO NOT USE WATER WHEN IT MAY CREATE HAZARDOUS OR OBJECTIONABLE CONDITIONS SUCH AS ICE, FLOODING, AND POLLUTION.
8. CLEANING AND MAINTENANCE: THE CONTRACTOR SHALL PROTECT EXISTING BUILDING FINISHES INDICATED TO REMAIN, AND SHALL CLEAN SURFACES SOILED AS A RESULT OF THE WORK OF THIS CONTRACT DAILY.

DRAWING NOTE KEY

DRAWING NOTES:

2.1.1 BUILDING FILL: CONTROLLED COMPACTED FILL. COMPLY WITH THE REQUIREMENTS OF THE GEOTECHNICAL REPORT PREPARED BY THE ATLAS ENGINEERING LAB DATED ****, 2025.

2.2.1 BUILDING BACKFILL: CONTOUR GRADES AWAY FROM THE BUILDING AT A 5% SLOPE. USE EXCAVATE NATIVE MATERIALS PROVIDE 4" OF TOPSOIL AND SEED. PROVIDE CONTROLLED COMPACTED FILL AT DRIVE AREAS

3.1.1 CONCRETE FOOTING: POURED IN PLACE CONCRETE FOOTING. REFER TO STRUCTURAL DRAWINGS AS PREPARED BY POST FRAME BUILDING MFGR FOR SIZE, DEPTH AND REINFORCING.

3.1.2 CONCRETE SLAB ON GRADE: REINFORCED CONCRETE SLABS ON GRADE. REFER TO STRUCTURAL DRAWINGS AS PREPARED BY POST FRAME BUILDING MFGR FOR THICKNESS AND REINFORCING

3.2.1 CONCRETE COLUMN PIER: REFER TO STRUCTURAL DRAWINGS AS PREPARED BY POST FRAME BUILDING MFGR FOR ADDITIONAL INFORMATION.

5.1.1 STEEL BOLLARD: CONCRETE FILLED, W/ PROTECTIVE COVER. REFER TO DETAIL, DRAWING A11

6.1.1 GLULAM WOOD COLUMN: COLUMN SIZING PROVIDED FOR REFERENCE ONLY. THE POST FRAME BUILDING CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ALL FRAME MEMBERS.

6.1.2 LAMINATED WOOD COLUMN: COLUMN SIZING PROVIDED FOR REFERENCE ONLY. THE POST FRAME BUILDING CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ALL FRAME MEMBERS.

6.2.1 LAMINATED BEAM / LINTEL: THE POST FRAME BUILDING CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL FRAME MEMBERS. SEE STRUCTURAL DRAWINGS AS PREPARED BY POST FRAME BUILDING MFGR.

6.2.2 DIMENSIONAL LUMBER BEAM / LINTEL: MEMBERS AS SHOWN 4 DETAILED ON THE CONTRACT DRAWINGS. POST FRAME BUILDING CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ALL FRAME MEMBERS.

6.2.3 LAMINATED WOOD BEAM: COLUMN SIZING PROVIDED FOR REFERENCE ONLY. THE POST FRAME BUILDING CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ALL FRAME MEMBERS.

6.3.1 PURLINS: ROOF / WALL FRAMING PURLINS, FABRICATED FROM DIMENSIONAL LUMBER AS SIZED AND SPACED BY THE POST FRAME BUILDING CONTRACTOR.

7.1.1 ROOF METAL: 26 GA. GALVALUME STEEL PANEL, METAL SALES "CLASSIC RIB" PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER. COLOR AS SELECTED BY OWNER.

7.1.2 EXTERIOR SOFFIT FINISH: 29 GA. GALVALUME STEEL SOFFIT PANEL, VENTED UNLESS NOTED OTHERWISE. PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER.

7.1.3 VENTED RIDGE CAP: 24 GA. GALVALUME STEEL VENTED RIDGE CAP, W/ INSECT SCREEN. SECURE TO ROOF PURLIN. W/ MASTIC & DOUBLE BEAD TAPE. PROVIDE PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER. SEE DETAIL 7/A11

7.1.4 ROOF EAVE TRIM: 24 GA. GALVALUME STEEL FORMED EAVE TRIM WITH DRIP. SECURE TO SOLID NAILERS AND ROOF PURLIN. INSTALL W/ MASTIC AND DOUBLE BEAD TAPE. PROVIDE PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER.

7.1.5 SNOW GUARD: 5S5 RIBBRACKET III COLORGARD SNOW RETENTION RAIL AND SNOW CLIP. W/ SEALANT TAPE. SPACE SNC CLIPS AT 12" O.C. COLOR STRIP TO MATCH ROOF. SEE DETAIL 5/A11.

7.2.1 EXTERIOR WALL FINISH: 29 GA. GALVALUME STEEL PANEL, METAL SALES "CLASSIC RIB" PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER. PROVIDE ALL REQ'D CLOSURE AND J TRIM, INT. AND EXT. CLOSURES AND FASTENERS.

7.2.2 EXTERIOR WALL TRIM: 24 GA. GALVALUME STEEL TRIM, DIMENSION AS NOTED. PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER. DRIP AT BOTTOM EDGE.

7.3.1 GUTTER / DOUNSPOUTS: 22 GA. 4" X 3" GALVALUME STEEL DOUNSPOUTS, EXTEND TO STORM WATER SYSTEM. COLOR AS SELECTED BY OWNER.

7.3.2 DRIP EDGE: 24 GA. GALVALUME STEEL DRIP EDGE AND RETAINER CLIP. PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER.

7.3.3 MASONRY CAP: 24 GA. GALVALUME STEEL MASONRY CAP / WATERTABLE AND RETAINER CLIP. EXTEND VERTICALLY BEHIND SIDING 4". PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER.

7.3.4 BRAKE METAL TRIM: 24 GA. GALVALUME STEEL FORMED BRAKE METAL TRIM SIZE AND CONFIGURATION AS DETAILED. PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER.

7.3.5 J-BEAD TRIM: 24 GA. GALVALUME STEEL FORMED J-BEAD TRIM SIZE AND CONFIGURATION AS DETAILED. PVDF/FEVE FINISH. COLOR AS SELECTED BY OWNER.

7.4.1 WEATHER BARRIER: .060" SELF-ADHERING, ASPHALT BASED HIGH TEMPERATURE WEATHER BARRIER UNDERLAYMENT OVER SUBSTRATE.

7.4.2 ROOF INSULATION: FOIL FACED R-38 BATT INSULATION INSTALLED ALONG THE BOTTOM CHORD OF THE TRUSSES.

7.4.3 EXTERIOR WALL INSULATION: FOIL FACED R-21 BATT INSULATION.

7.4.4 INTERIOR WALL INSULATION: NOT APPLICABLE.

8.1.1 ENTRANCE DOOR: PUBLIC ALUMINUM STOREFRONT ENTRANCE, REFER TO DOOR SCHEDULE. , DRAWING A11.

8.1.2 ACCESS DOOR: SINGLE DOOR, REFER TO DOOR SCHEDULE, DRAWING A11.

8.2.1 SECTIONAL OVERHEAD DOOR SERVICE: MANUALLY OPERATED OVERHEAD DOOR, REFER TO DRAWING A11.

8.3.1 EXTERIOR WINDOW: REFER TO WINDOW SCHEDULE, DRAWING A12 FOR SIZE AND WINDOW DETAILS

9.1.1 INTERIOR FLOOR FINISH, MAINTENANCE AND STORAGE ROOMS: URETHANE FLOOR FINISH, REFER TO ROOM FINISH SCHEDULE, A12

9.1.2 INTERIOR FLOOR FINISH, BASE BID, TASTING ROOM: STAIN AND SEAL FLOOR FINISH, REFER TO ROOM FINISH SCHEDULE, A12

9.1.3 INTERIOR FLOOR FINISH, LVT: LVT FLOOR FINISH, PATCRAFT "TIMBER GROVE II" OR EQUAL.

9.1.4 INTERIOR FLOOR BASE: 4" H. VINYL COVE BASE. SEAL JOINT AT FLOOR WHERE SCHEDULED. PROVIDE IN 120 FT. LENGTHS. INSTALL WITHOUT JOINTS.

9.2.1 INTERIOR WALL FINISH, MAINTENANCE AND STORAGE ROOMS, METAL: 29 GA. METAL LINER PANEL, COLOR: STANDARD WHITE.

9.2.2 INTERIOR WALL FINISH, TOILET ROOMS: PROVIDE 5/8" TYPE CEMENT WALLBOARD (FIBEROCK, AQUAROC OR EQUAL) AT ALL BATHROOMS.

9.3.1 INTERIOR CEILING FINISH, MAINTENANCE AND STORAGE ROOMS, METAL: 29 GA. METAL LINER PANEL, COLOR: STANDARD WHITE.

9.3.2 INTERIOR CEILING FINISH GUB: ½" GYPSUM WALL BOARD, FINISH AS SCHEDULED. ⅝" GUB WHERE SCHEDULED AND AT FIRE RATED ASSEMBLIES.

10.1.1 WALL MOUNTED CANOPY: WALL MOUNTED CANOPY, 9'-6" WIDE, 3'-0" DEEP, WITH 2" X 2" DOUNPOUT TO GRADE, AS MANUFACTURED BY "MAPES", OR EQUAL.

10.2.1 BUILDING MOUNTED WALL SIGNAGE: CUSTOM BUILDING MOUNTED WALL ILLUMINATED SIGNAGE, COLOR AS SELECTED BY OWNER

23.1.1 MECHANICAL: INTAKE / EXHAUST LOUVER, REFER TO MECHANICAL DRAWINGS FOR SIZE. PROVIDE WITH HEAD FLASHING, DRIP.

26.1.1 ELECTRICAL: WALL MOUNTED LIGHTING, REFER TO ELECTRICAL DRAWINGS FOR LIGHTING FIXTURE SCHEDULE

DATE	No.	REVISION
10/30/26	1	RELEASE FOR BID

LOT NO. 3
BURLINGTON COUNTY

LRHSD PROJECT NO. REF21-43-8319, 2-010-24
PROPOSED NEW MAINTENANCE FACILITY

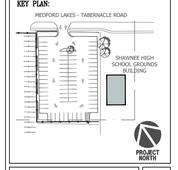
SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING

580 TABERNACLE ROAD, MEDFORD, NEW JERSEY 08055
MEDFORD TOWNSHIP
BLOCK NO. 4704

Linda M. Vivenzi, AIA, Secretary/Treasurer
New Jersey Architect LIC # 21GI00635000

Drew C. Dingler, AIA, President
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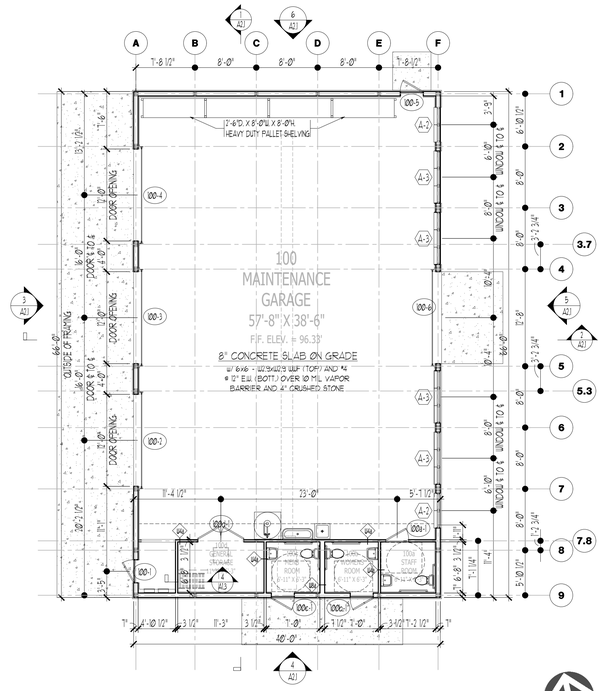
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SCALE:	AS NOTED	DRAWN BY:	DCD
COMM. No.	8319	CHECKED BY:	DCD
DATE:	11/1/2024		

SHEET TITLE:
GENERAL NOTES & DRAWING KEY

SHEET No.:
G1.0

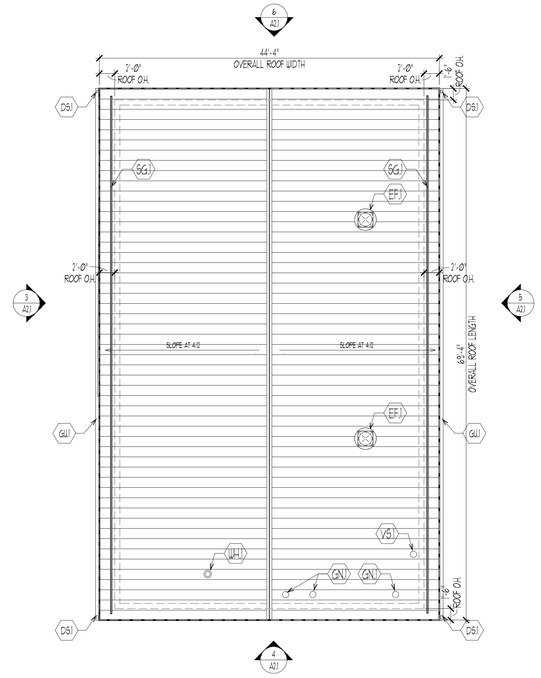
FILE G:\PROJECTS\8319\ARCHITECT\MAU-A1-AB SERIES\8319 G0.0 COVER SHEET.DWG DATE 11/5/2025 4:12 PM USER drew dingler



1 A1.1 FIRST FLOOR PLAN MAINTENANCE GARAGE

1/8"=1'-0"

PROJECT NORTH



3 A1.1 ROOF PLAN MAINTENANCE GARAGE

1/8"=1'-0"

PROJECT NORTH

EXTERIOR RAIN WATER LEADERS - RECTANGULAR

	Type	Width	Depth	Free Area	Allowable Roof Area, NSPC Table 13.6.1 (*)	Comment
DS	3" X 4" RECT.	3.00 In.	4.00 In.	12.00 sq. in.	2162.00 sq. ft.	.050 Galvalume

(*) ALLOWABLE SIZE OF VERTICAL STORM WATER CONDUCTORS BASED ON ROOF AREA. PER THE TABLE ABOVE, ALLOWABLE ROOF AREA VALUES ARE AS INTERPOLATED FROM NSPC TABLE 13.6.1. VALUES, WHICH MEET OR EXCEED THE REQUIREMENTS OF THE AREAS BEING SERVED.

ALL DOWNSPOUTS SHALL BE CONNECTED TO THE UNDERGROUND STORM WATER SYSTEM. PROVIDE J.R. HOE DOWNSPOUT BOOT AT ALL DOWNSPOUTS. PROVIDE MODEL NO. O-3424B, 24" LENGTH WITH CLEANOUT, BELL SIZED TO ACCOMMODATE 3" X 4" DOWNSPOUTS.

GUTTER SIZING:
ALLOWABLE SIZE OF GUTTERS AT STANDING SEAM ROOF AREAS IS BASED ON THE 2021 NATIONAL STANDARD PLUMBING CODE (NSPC), CHAPTER 13, AND THE NATIONAL INSTITUTE OF STANDARDS AND TECHNOLOGY (NIST) BASED ON THE FOLLOWING FORMULA:

WHERE: $A = {}^{4/7}\,I^{3/28}\,(L)^{5/14}\,M$

A = AREA TO BE DRAINED (CALCULATIONS BASED ON A MAXIMUM OF 1,650 S.F.)
I = RAINFALL INTENSITY IN INCHES / HOUR (6 INCHES /HOUR PER N.J.A.C. 5:23-3.15)
L = LENGTH OF GUTTER IN FEET (CALCULATIONS BASED ON A MAXIMUM OF 30 FT.)
M = RATION OF GUTTER DEPTH TO WIDTH (0.8333 ASSUMED)

BASED ON THE ABOVE VALUES, A MAXIMUM GUTTER WIDTH OF 2.72 INCHES IS REQUIRED. GUTTERS SHALL BE 3" X 4"

GU DESIGNATES 5" ALUM. "K" STYLE GUTTER, 0.032" GAUGE, BAKED ENAMEL FIN., W/ STRAP HANGERS

EF DESIGNATES ROOF MOUNTED EXHAUST FAN. REFER TO MECH. DRAWINGS FOR ADDITIONAL INFORMATION.

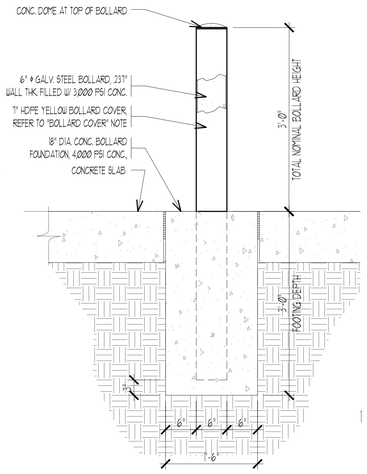
GN DESIGNATES MECHANICAL GOOSENECK EXHAUST. REFER TO MECH. DRAWINGS FOR ADDITIONAL INFORMATION.

SG DESIGNATES NEW S!5 SNOW GUARD, BRACKETS @ 16" O.C., ALIGN W/ STRUCTURE, SEE DETAILS THIS SHEET.

VS DESIGNATES PLUMBING VENT STACK. REFER TO PLUMBING DRAWINGS FOR ADDITIONAL INFORMATION.

WH DESIGNATES WATER HEATER EXHAUST. REFER TO PLUMBING DRAWINGS FOR ADDITIONAL INFORMATION.

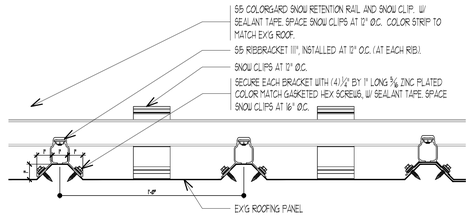
Know what's below. Call before you dig.



4 A1.1 BOLLARD DETAIL TYP. FOR (8)

1"=1'-0"

BOLLARD COVER NOTE:
1. MANUFACTURER: PROVIDE PRODUCTS AS MANUFACTURED BY "POST GUARD" ENCORE COMMERCIAL PRODUCTS, INC. OR APPROVED EQUAL.
2. MATERIALS: HDPE AND LDPE POLYETHYLENE, .075 INCH NOMINAL WALL THICKNESS. FLEXURAL MODULUS: 100,000 PSI, TENSILE STRENGTH: 4,000PSI
3. SIZE: (T)DIAMETER X 48"LONG+) CONTRACTOR TO VERIFY LENGTH, AND CUT AS REQUIRED. SLEEVE HEIGHT WILL BE FROM THE TOP OF THE SURROUNDING PAVEMENT TO THE TOP OF THE EXPOSED PIPE.
4. COLOR: YELLOW
5. TAPE COLOR: RED 3M SERIES REFLECTIVE TAPE, W/ MOLDED RECESS 5.875 INCHES APART
6. INSTALLATION TO BE IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS.



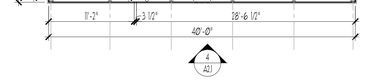
SCALE: 3" = 1'-0"

SCALE: FULL SIZE

5 A1.1 SNOW GUARD MAINTENANCE BUILDING

3"=1'-0"

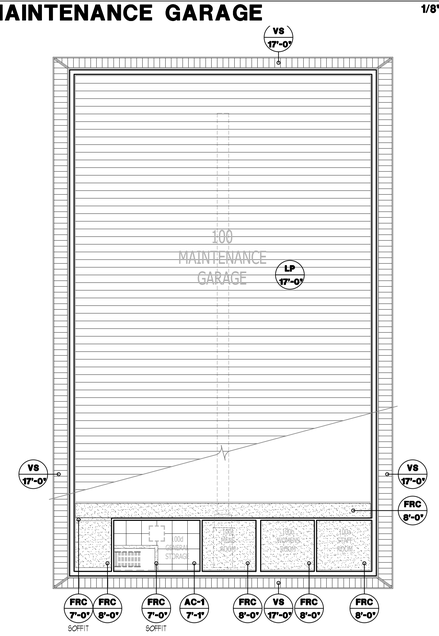
1. SCREWS WITH WASHERS AND GASKETS SHALL BE PROVIDED AT ALL EXPOSED FASTENER LOCATIONS.
2. SNOW GUARDS SHALL BE AS MFGR'D BY S!5, NO EXCEPTIONS PERMITTED. SNOW GUARD BRACKETS SHALL BE "RIBBRACKET III", INSTALLED AT 12" O.C. (AT EACH RIB). SECURE EACH BRACKET WITH (4) 1/4" BY 1" LONG 5/16 ZINC PLATED COLOR MATCH GASKETED HEX SCREWS.
3. SECURE NEW S!5 COLORGARD SNOW RETENTION RAIL AND SNOW CLIP TO SNOW GUARD BRACKET. SPACE SNOW CLIPS AT 12" O.C. COLOR STRIP TO MATCH EX'G ROOF.



1a A1.1 MECHANICAL EQUIPMENT PLATFORM PLAN MAINTENANCE GARAGE

1/8"=1'-0"

PROJECT NORTH



KEY: R.C.P.	
	2' x 4' LIGHT FIXTURE
	2' x 2' LIGHT FIXTURE
	RETURN OR EXHAUST GRILL. SIZES VARY FROM 6X6 TO 20X18
	SUPPLY DIFFUSSER
	GWB SOFFIT OR CEILING

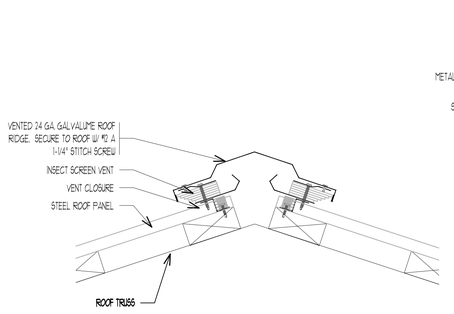
GENERAL CEILING NOTES:

1. REFER TO MECHANICAL / PLUMBING, AND ELECTRICAL DRAWINGS FOR FULL EXTENT OF THESE SYSTEMS.
2. THE LINER PANEL CEILING IS TO BE INSTALLED WITH 2" X 4" FURRING AT 16" O.C. AT THE UNDERSIDE OF THE TRUSSES THROUGHOUT THE ENTIRETY OF THE BUILDING.
3. "FRC" REFERS TO ¾" FIBERGLASS REINFORCED WALL BOARD.
4. "LP" REFERS TO "EVERLAST" LINER PANEL. WHITE FINISH.
5. "VS" REFERS TO "EVERLAST" VENTED EXTERIOR SOFFIT SYSTEMS

AC-# — DESIGNATES CEILING FINISHES
8'-0" — DESIGNATES CEILING HEIGHT

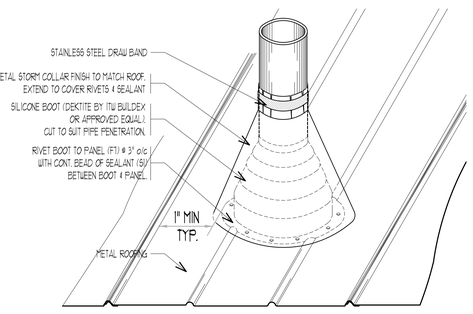
2 A1.1 REFLECTED CEILING PLAN MAINTENANCE GARAGE

1/8"=1'-0"



7 A1.1 ROOF RIDGE CAP

3"=1'-0"



6 A1.1 DETAIL VENT FLASHING

N.T.S.

10/30/26	1	RELEASE FOR BID
DATE	No.	REVISION

LOT NO. 3
BLOCK NO. 4704

LRHSD PROJECT NO. REF21-43-8319, 2-010-24
PROPOSED NEW MAINTENANCE FACILITY

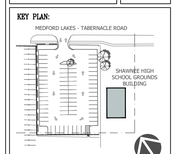
SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING

580 TABERNACLE ROAD, MEDFORD, NEW JERSEY 08055
MEDFORD TOWNSHIP BURLINGTON COUNTY

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COMM. No.	8319	CHECKED BY: DCD
DATE:	11/1/2024	

SHEET TITLE:
FLOOR PLANS, ROOF PLAN AND SCHEDULES

SHEET No.: A1.1

FILE: G:\PROJECTS\8319\ARCHITECTURAL\A1-A8 SERIES\A1|8319 A1.1 FLOOR PLAN.DWG DATE: 11/5/2026 11:24 AM USER: drew dingler

ROOM FINISH SCHEDULE - SHAWNEE HIGH SCHOOL - MAINTENANCE BUILDING

ROOM NO.	ROOM NAME	FLOOR				BASE			WALL			WAINSCOT				CEILING				REMARKS
		MAT.	FINISH	TYPE	COLOR	MAT.	HT.	COLOR	MAT.	FINISH	COLOR	MAT.	FINISH	COLOR	HT.	MAT.	FINISH	COLOR	HT.	
100	MAINTENANCE BAY	CONCRETE	EPOXY	N/A	BY OWNER	VINYL	4"	BY OWNER	LINER PANEL	PRE-FINISH	WHITE	N/A				LINER PANEL	PRE-FINISH	WHITE	17'-0"	
100a	STAFF ROOM	CONCRETE	EPOXY	N/A	BY OWNER	VINYL	4"	BY OWNER	FRC BOARD	SP COAT	BY OWNER	FRP	FRP	BY OWNER	4'-0"	FRC BOARD	SP COAT	BY OWNER	8'-0"	
100b	WOMENS ROOM	CONCRETE	EPOXY	N/A	BY OWNER	VINYL	4"	BY OWNER	FRC BOARD	SP COAT	BY OWNER	N/A	FRP	BY OWNER	4'-0"	FRC BOARD	SP COAT	BY OWNER	8'-0"	
100c	MENS ROOM	CONCRETE	EPOXY	N/A	BY OWNER	VINYL	4"	BY OWNER	FRC BOARD	SP COAT	BY OWNER	N/A	FRP	BY OWNER	4'-0"	FRC BOARD	SP COAT	BY OWNER	8'-0"	
100d	GENERAL STORAGE	CONCRETE	EPOXY	N/A	BY OWNER	VINYL	4"	BY OWNER	FRC BOARD	SP COAT	BY OWNER	N/A				AC-1	PRE-FINISH	WHITE	T-1*	
100	EQUIPMENT PLATFORM	ENG. W.P.	RUBBER FLR	N/A	BLACK	VINYL	4"	BLACK	LINER PANEL	PRE-FINISH	BY OWNER	N/A				LINER PANEL	PRE-FINISH	WHITE	17'-0"	
100a	PLATFORM ACCESS	ENG. W.P.	RUBBER FLR	N/A	BLACK	VINYL	4"	BLACK	FRC BOARD	SP COAT	BY OWNER	N/A				LINER PANEL	PRE-FINISH	WHITE	8'-2"	

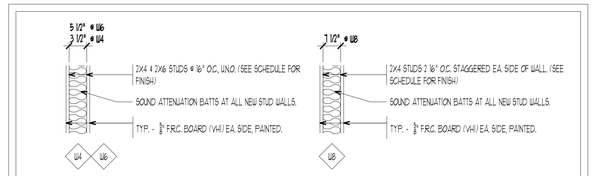
ROOM FINISH SCHEDULE NOTES:
1. EQUIPMENT PLATFORM SUBFLOOR 23/32" ENGINEERED WOOD STRUCTURAL PANEL (ENG. SUB.) "ADVANTECH SUBFLOORING" BY HUBER ENGINEERED WOODS, OR EQ.
2. EQUIPMENT PLATFORM FINISH FLOOR TO BE DIAMOND PLATE ROLL STYRENE BUTADIENE RUBBER (SBR) MATTING BY RUBBER-CAL, INC. LENGTH: 50FT, WIDTH: 48", THICKNESS: 1/8", HARDNESS: 60 SHORE A NOMINAL, COMPRESSION: 36% @ 50 PSI, TENSILE STRENGTH: 600 PSI
3. PROVIDE 5/8" FIBER REINFORCED CEMENT WALLBOARD UNO, FINISHED AS NOTED
4. PAINTING: ALL EXPOSED UNFINISHED SURFACES TO BE PAINTED WITH A PRIMER COAT PLUS 2 FINISH COATS.
   a. ALL NEW INTERIOR WALLS TO BE PRIMED THEN PAINTED WITH TWO COATS PAINT UNLESS NOTED OTHERWISE BY OWNER.
3. FINISHED FLOORING: APPLIED FINISHED FLOORING SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
   a. FLAMMABILITY: RATING: PASSING METHENAMINE PILL TEST
   b. TEST METHOD: ASTM D 2859, OR DOC FF-1-70
4. FLAME SPREAD AND SMOKE DEVELOPED INDEXES FOR WALLS AND CEILING FINISHES SHALL BE IN ACCORDANCE WITH 2018 IBC-NJ 803 AS FOLLOWS. MATERIALS SHALL BE TESTED IN ACCORDANCE WITH ASTM E84 OR UL 723.
   a. FLAME SPREAD - 1200 MAX.
   b. SMOKE DEVELOPED - 450 MAX.
5. COLORS SHALL BE SELECTED BY THE OWNER.

DOOR SCHEDULE - SHAWNEE HIGH SCHOOL - MAINTENANCE BUILDING

DOOR NO.	DOOR SIZE	DOOR TYPE	FRAME TYPE	FRAME DEPTH	DOOR			FRAME			GLASS	DETAIL			THRESHOLD	HRDWR. SET	REMARKS
					MAT.	FIN.	LABEL	MAT.	FIN.	LABEL		HEAD	JAMB	SILL			
100-1, 100-5	(1) 3'-0" x 7'-0" x 1-3/4"	N-1	Ø1	7-1/4"	FRP	FRP	N/A	ALUMINUM	CLEAR ANOD.	N/A	G-A	ED1	ED1	ED5	ALUMINUM	Ø2	
100-2, 100-3, 100-4 100-6	(1) 12'-0" x 12'-0" x 1"	OH-1	TRM OPG	N/A	INSUL. STEEL	PREFINISHED	N/A	TRM OPG	TRM OPG	N/A	G-OH-A	OH1	OH1	OH5	N/A	Ø1	
100a-1	(2) 3'-0" X 6'-8" X 1-3/4"	F-1	Ø1	7-1/4"	INSUL. H.M.	FTD	N/A	H.M.	FTD	N/A	N/A	ID1	ID1	ID5	N/A	Ø3	
100b-1	(1) 3'-0" x 7'-0" x 1-3/4"	F-1	Ø1	7-1/4"	FRP	FRP	N/A	ALUMINUM	CLEAR ANOD.	N/A	N/A	ED1	ED1	ED5	ALUMINUM	Ø4	
100c-1	(1) 3'-0" x 7'-0" x 1-3/4"	F-1	Ø1	7-1/4"	FRP	FRP	N/A	ALUMINUM	CLEAR ANOD.	N/A	N/A	ED1	ED1	ED5	ALUMINUM	Ø4	
100d-1	(2) 3'-0" x 6'-8" x 1-3/4"	N-1	Ø2	7-1/4"	INSUL. H.M.	FTD	N/A	H.M.	FTD	N/A	G-B	ID1	ID1	ID5	N/A	Ø5	
100a-1	3'-0" X 6'-8" X 1-3/4"	N-1	Ø1	7-1/4"	INSUL. H.M.	FTD	N/A	H.M.	FTD	N/A	G-B	ID1	ID1	ID5	N/A	Ø6	

GENERAL DOOR SCHEDULE NOTES:
1. REFER TO DOOR HEAD, JAMB AND SILL DETAILS ON DRAWING A12 FOR ADDITIONAL INFORMATION.
2. FIGURES WITHIN PARENTHESIS PRECEDING THE DOOR SIZE (*) INDICATES THE NUMBER OF DOOR LEAVES.
3. FIGURES WITHIN PARENTHESIS FOLLOWING THE DOOR NUMBER (*) INDICATES THE QUANTITY OF DOOR LOCATIONS. REFER TO THE FLOOR PLANS
4. DOOR AND ALUMINUM FINISH TO BE COLOR ANODIZED. ALLOW FOR (2) SELECTIONS FOR BOTH DOORS AND FRAMES.
5. IMPACT FILM SHALL BE AVERY-DENNISON "SF CLEAR 7 MIL" WHERE IMPACT RESISTANT SECURITY FILM IS INDICATED WITHIN THE GLAZING NOTES BELOW.

GLAZING SCHEDULE:
G-A: 1" INSULATED EXTERIOR LITE: 1/4" CLEAR TEMP. LOW-E 1" SURFACE INTERIOR LITE: 1/4" CLEAR TEMP.
G-B: 1/4" CLEAR TEMP.

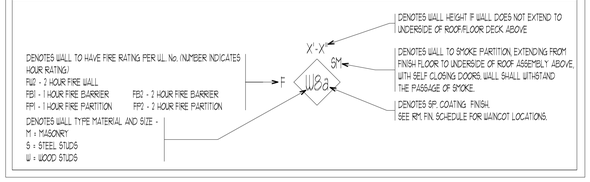


GENERAL NOTES - WALL TYPES AND FINISH TYPES

SCALE : 1" = 1'-0"

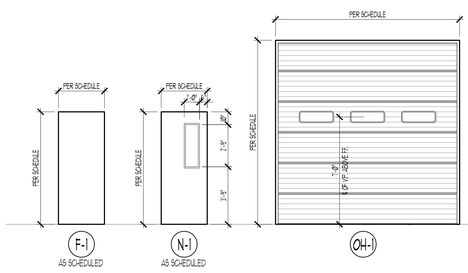
1. FINISHES SHALL BE AS NOTED, UNLESS SPECIFICALLY INDICATED OTHERWISE ON ROOM FINISH SCHEDULE.
2. WALLS SHALL EXTEND FROM FINISH FLOOR TO UNDERSIDE OF ROOF DECK OR FLOOR ABOVE UNLESS NOTED OTHERWISE.

WALL TYPE SYMBOL



ABBREVIATION LIST

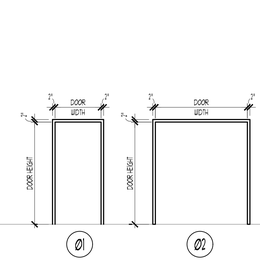
AC-	ACOUSTIC CEILING PANEL		
CER. TILE	CERAMIC TILE	MAT.	MATERIAL
CLG.	CEILING	MFGR	MANUFACTURER
CMU	CONCRETE MASONRY UNIT	MR	MOISTURE RESISTANT
CONC.	CONCRETE	N/A	NOT APPLICABLE
EX'G	EXISTING	PTD	PAINTED
EXP.	EXPOSED (CONSTRUCTION)	PTD ACC'T	PAINTED ACCENT
FIN.	FINISH	RCB	RUBBER COVE BASE
FL.	FLOOR	SP. COAT	SPECIAL COATING FINISH
FLR. CT.	FLOOR COATING	STLS STL	STAINLESS STEEL
FR	FIBERGLASS REINFORCED	SVF	SOLID VINYL FLOOR
FRC	FIBERGLASS REINFORCED CORE	T.P.F.	TRANSPARENT FINISH
FRP	FIBERGLASS REINFORCED PANELS	VCT	VINYL COMPOSITION TILE
GWB	GYPSUM WALL BOARD		
HT.	HEIGHT		



DOOR TYPES

1/4" = 1'-0"

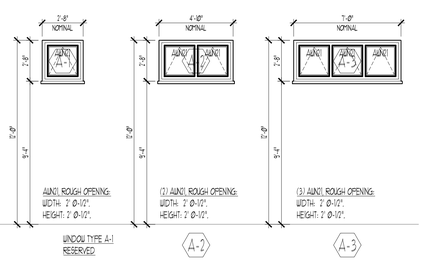
SEE DOOR SCHEDULE FOR MATERIAL, GLAZING & DIMENSIONS NOT NOTED.



FRAME TYPES

1/4" = 1'-0"

SEE DOOR SCHEDULE FOR MATERIAL, GLAZING & DIMENSIONS NOT NOTED.



WINDOW TYPES

1/4" = 1'-0"

WINDOW TYPES:
WINDOW TYPES A-1, A-2, AND A-3 SHALL BE AWNING VINYL WINDOWS W/ "THERMAGUARD" LOW-E INSULATED GLASS, BY HARVEY WINDOWS AND DOORS, 1400 MAIN STREET, WALTHAM, MA 01451-1689. 1-800-822-8437

DATE	No.	REVISION
10/30/26	1	RELEASE FOR BID

LOT NO. 3

LRHSD PROJECT NO. REF21-43-8319, 2-010-24

PROPOSED NEW MAINTENANCE FACILITY

SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING

580 TABERNACLE ROAD, MEDFORD, NEW JERSEY 08055

BLOCK NO. 4704

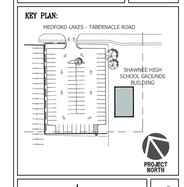
MEDFORD TOWNSHIP

BURLINGTON COUNTY

Linda M. Vivenzi, AIA, Secretary/Treasurer
New Jersey Architect Lic.# 21AI00060300 P.P. Lic.# 33UA00650300

Drew C. Dingler, AIA, President
New Jersey Architect Lic.# 21AI01088500

KEY PLAN:



THE DESIGN COLLABORATIVE
ARCHITECTS AND PLANNERS, P.A.
1940 ROUTE 9 NORTH
CAPE MAY COURT HOUSE, NEW JERSEY 08210
PHONE: 609 465 4111 FAX: 609 465 9238
WWW.TDCARCHITECT.COM

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SCALE:	AS NOTED	DRAWN BY:	DCD
COMM. No.	8319	CHECKED BY:	DCD
DATE:	11/1/2024		

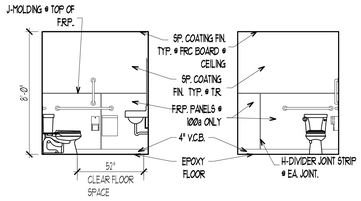
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SCHEDULES & DETAILS

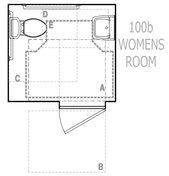
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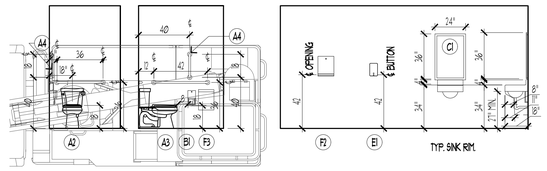
A1.2

FILE C:\PROJECTS\8319\ARCHITECTURAL\A1-A8 SERIES\A1\8319 A1.1 FLOOR PLAN.DWG DATE 11/5/2026 11:23 AM USER drew dingler



#	LOCATION	W	L	CITATION
A	DOOR PUSH	36	48	Fig 404.2.3.2(D
B	DOOR PULL	60	48	Fig 404.2.3.2(G)
C	WC	60	56	Fig. 604.3
D	Sink	30	52	606.2, Fig. 305.3.1
E	TURNING SPACE	68	60	304.3.2,Fig. 304.3.2.1(A)





TOILET ROOM ACCESSORY SCHEDULE

RM. NO.	ROOM NAME	GRAB BARS A1	GRAB BARS A2	GRAB BARS A3	GRAB BARS A4	TOILET TISSUE DISPENSER B1	MIRROR C1	SOAP DISPENSER E1	WASTE F1	TOWEL DISPENSER F2	SAN. NAP. DISPOSAL F3
		B-6106.99	B-6106.99	B-6106.99	B-6106.99	OWNER SUPPLIED	B-790	OWNER SUPPLIED	OWNER SUPPLIED	OWNER SUPPLIED	B-354
		24"	36"	42"	18"					EQT. 48" AFF.	SURFACE MOUNTED
100a	STAFF ROOM		●	●	●	●	●	●	●	●	●
100b	WOMENS ROOM		●	●	●	●	●	●	●	●	●
100c	MENS ROOM		●	●	●	●	●	●	●	●	●

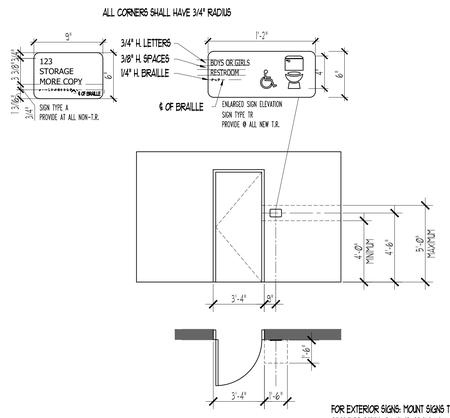
1 A1.3 INTERIOR ELEVATION 100A, 100B.

1/4"=1'-0"

100C OPP. HAND

2 A1.3 TOILET ROOM ACCESSORIES MOUNTING HEIGHTS & REQU. CLEARANCES

1/4"=1'-0"



NOTE: COLOR, FONT AND SIGN FINISH TO MATCH EXISTING BUILDING SIGNAGE.

GENERAL NOTES FOR INTERIOR AND EXTERIOR SIGNAGE:

1. LETTERS AND NUMBERS ON SIGN SYSTEM SHALL HAVE THE FOLLOWING:
   - A. WIDTH-TO-HEIGHT RATIO OF BETWEEN 3:5 AND 1:1
   - B. STROKE WIDTH-TO-HEIGHT RATIO OF BETWEEN 1:5 AND 1:10
   - C. CONTRAST IN VALUE WITH THE SIGN BACKGROUND UTILIZING LIGHT LETTERS ON A DARK BACKGROUND OR DARK LETTERS ON A LIGHT BACKGROUND.
   - D. MATTE FINISH ON A MATTE FINISH BACKGROUND.
2. CHARACTERS ON SIGNS SHALL BE:
   - A. RAISED FROM THE SIGN FACE AT LEAST 1/32"
   - B. BE BETWEEN 5/8" AND 2" HIGH
   - C. BE BLOCK TYPE OR SANS SERIF
   - D. BE ALL CAPITAL LETTERS
   - E. BE ACCOMPANIED BY GRADE II BRAILLE WITH A DOMED OR ROUNDED SHAPE
3. CHARACTERS AND GRADE II BRAILLE ARE TO BE INTEGRAL WITH THE SIGN FACE, NOT SEPARATELY ADHERED TO THE SIGN FACE, APPLIED AS A SEPARATE SURFACE, OR RECESSED INTO THE SIGN. TACTILE LETTERS AND NUMBERS MUST BE A MINIMUM OF 5/8" HIGH, AND A MAXIMUM OF 2" HIGH. LETTERS AND NUMBERS MUST BE RAISED A MINIMUM OF 1/32" FROM THE SIGN FACE.
4. ALL CORNERS SHALL BE RADIUSED 3/4"

GENERAL NOTES FOR SIGNAGE SUBMITTALS:

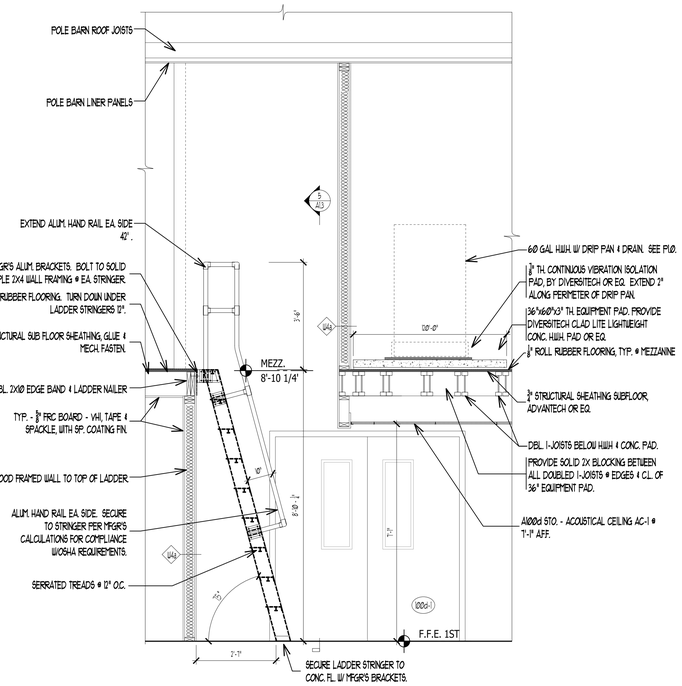
1. SUBMIT PRODUCT DATA INCLUDING MANUFACTURER'S CONSTRUCTION DETAILS RELATIVE TO MATERIALS, DIMENSIONS OF INDIVIDUAL COMPONENTS, PROFILES, AND FINISHES FOR EACH TYPE OF SIGN INDICATED WITHIN THE DETAILS ABOVE.
2. SUBMIT SHOP DRAWINGS FOR THE FABRICATION AND ERECTION OF SIGNS. SHOP DRAWINGS SHALL INCLUDE PLANS, ELEVATIONS, AND LARGE-SCALE SECTIONS OF TYPICAL MEMBERS AND OTHER COMPONENTS. SHOW ANCHORS, GROUNDS, REINFORCEMENT, ACCESSORIES, LAYOUT, AND INSTALLATION DETAILS (WHETHER OR NOT THE SIGN CONTRACTOR PERFORMS INSTALLATION OF SIGNS).

FOR EXTERIOR SIGNS: MOUNT SIGNS TO WALL W/ SILICONE & STAINLESS STEEL SCREWS AT EACH CORNER.

3 A1.3 SIGNAGE DETAILS

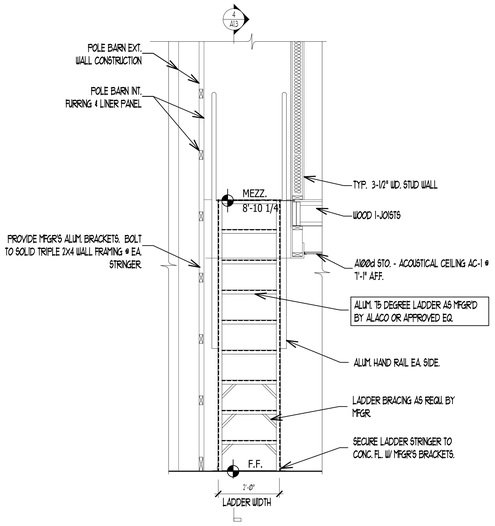
PROVIDE SIGNS AT THE FOLLOWING ROOMS: 100a STAFF ROOM; 100b WOMENS ROOM; 100c MENS ROOM; 100d GENERAL STORAGE; 120 EQUIPMENT PLATFORM

1/4"=1'-0"



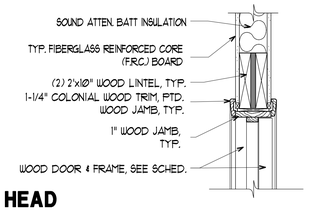
4 A1.3 LADDER SECTION HOT WATER HEATER PAD

1/2"=1'-0"

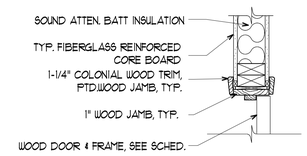


5 A1.3 PARTIAL SECTION LADDER ELEVATION

1/2"=1'-0"

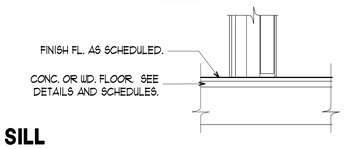


HEAD



JAMB

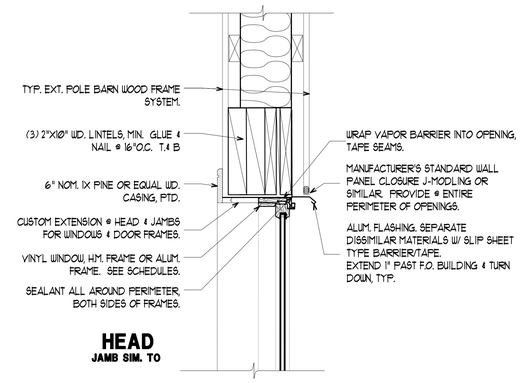
NOTE: PROVIDE SEALANT @ BOTH SIDES OF DOOR FRAMES TYP. FOR INTERIOR & EXTERIOR DOORS.



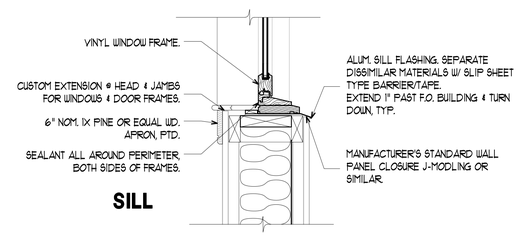
SILL

6 A1.3 DOOR DETAILS INTERIOR

1-1/2"=1'-0"



HEAD
JAMB SIM. TO



SILL

7 A1.3 WINDOW DETAILS EXT. DOORS , SIM. TO.

1-1/2"=1'-0"

DATE	No.	REVISION
10/30/26	1	RELEASE FOR BID

LOT NO. 3
BLOCK NO. 4704

LRHSD PROJECT NO. REF21-43-8319, 2-010-24
PROPOSED NEW MAINTENANCE FACILITY
SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING
680 TABERNACLE ROAD, MEDFORD, NEW JERSEY 08055
MEDFORD TOWNSHIP BURLINGTON COUNTY

Linda M. Vivenzi, AIA, Secretary/Treasurer
New Jersey Architect Lic.# 21AI00060000 P.P. Lic.# 33UA00650300

Drew C. Dingler, AIA, President
New Jersey Lic.# 21AI00588000

KEY PLAN:
MEDFORD LAKES - TABERNACLE ROAD
SHAWNEE HIGH SCHOOL GROUNDS BUILDING
PROJECT NORTH

THE DESIGN COLLABORATIVE
ARCHITECTS AND PLANNERS, P.A.
1940 ROUTE 9 NORTH
CAPE MAY COURT HOUSE, NEW JERSEY 08210
PHONE: 609 465 4111 FAX: 609 465 9358
WWW.TDCARCHITECT.COM

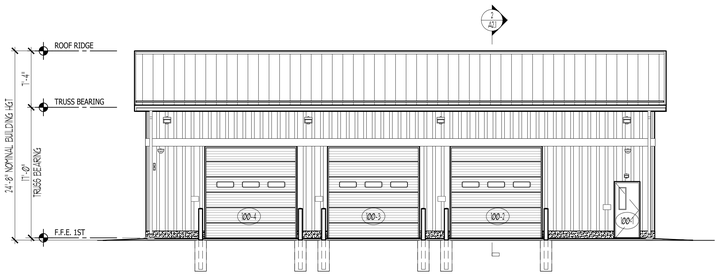
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SCALE:	AS NOTED	DRAWN BY:	DCD
COMM. No.	8319	CHECKED BY:	DCD
DATE:	11/1/2024		

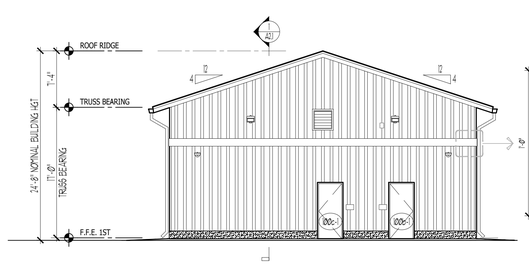
SHEET TITLE:
ACCESSORIES AND DETAILS

SHEET No.:
A1.3

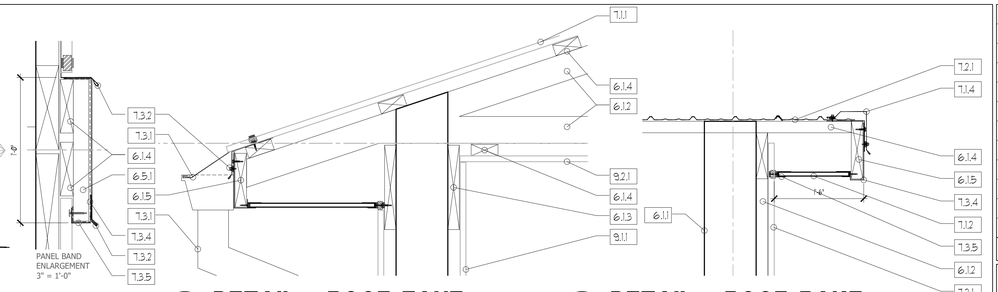
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3 A2.1 WEST ELEVATION MAINTENANCE GARAGE 1/8"=1'-0"

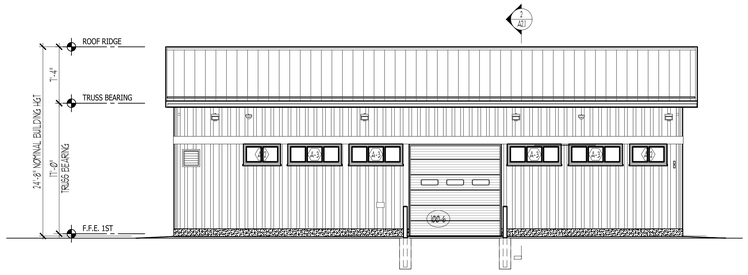


4 A2.1 SOUTH ELEVATION MAINTENANCE GARAGE 1/8"=1'-0"

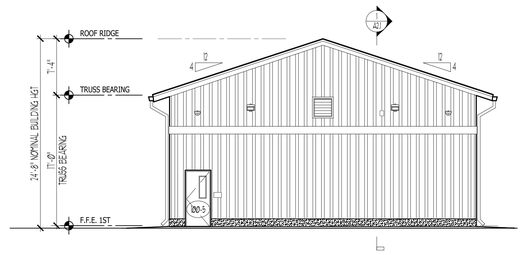


9 A2.1 DETAIL - ROOF EAVE MAINTENANCE GARAGE 1-1/2"=1'-0"

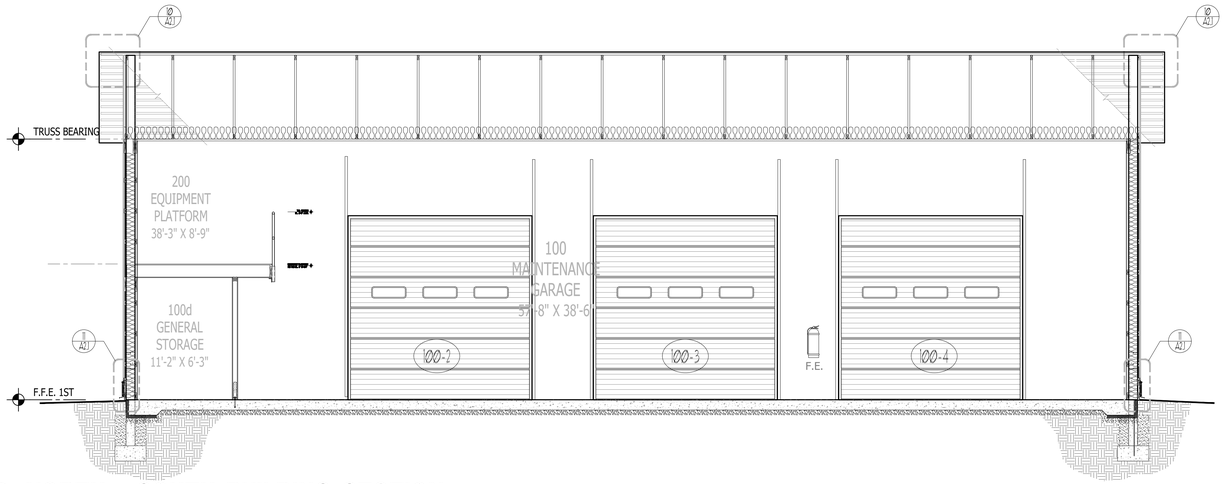
10 A2.1 DETAIL - ROOF RAKE MAINTENANCE GARAGE 1-1/2"=1'-0"



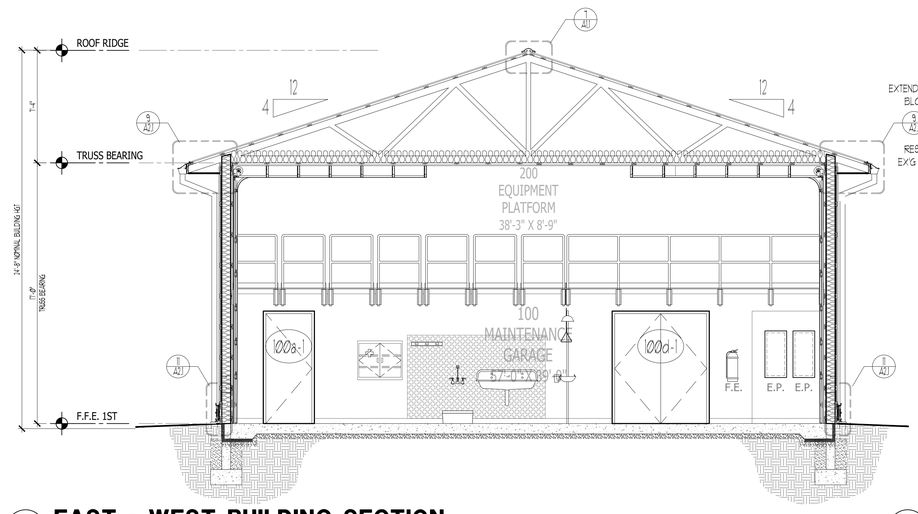
5 A2.1 EAST ELEVATION MAINTENANCE GARAGE 1/8"=1'-0"



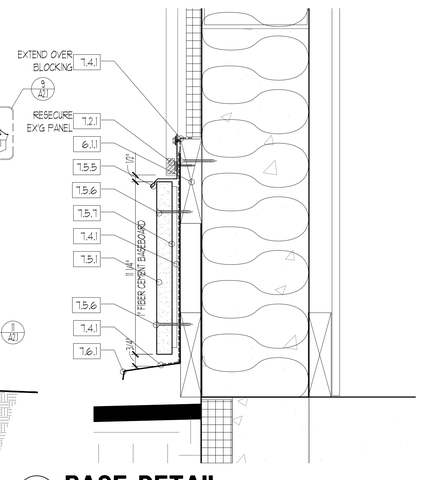
6 A2.1 NORTH ELEVATION MAINTENANCE GARAGE 1/8"=1'-0"



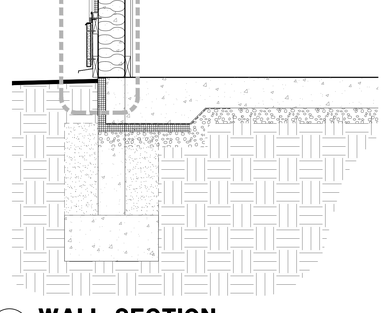
1 A2.1 NORTH - SOUTH BUILDING SECTION MAINTENANCE GARAGE 1/4"=1'-0"



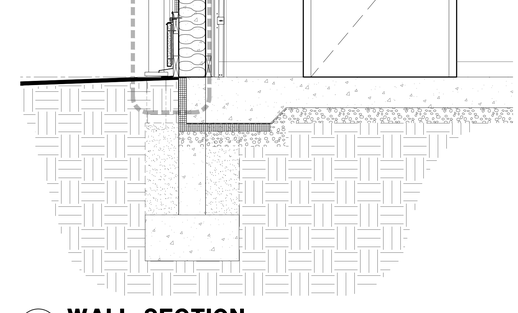
2 A2.1 EAST - WEST BUILDING SECTION MAINTENANCE GARAGE 1/4"=1'-0"



11 A2.1 BASE DETAIL MAINTENANCE GARAGE 3"=1'-0"



7 A2.1 WALL SECTION MAINTENANCE GARAGE 3/4"=1'-0"



8 A2.1 WALL SECTION MAINTENANCE GARAGE 3/4"=1'-0"

DATE	No.	REVISION
10/30/26	1	RELEASE FOR BID

LRHSD PROJECT NO. REF21-43-8319, 2-010-24
PROPOSED NEW MAINTENANCE FACILITY

SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING

580 TABERNACLE ROAD, MEDFORD, NEW JERSEY 08055

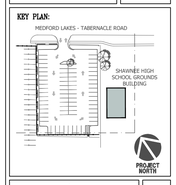
BLOCK NO. 4704 LOT NO. 3

MEDFORD TOWNSHIP BURLINGTON COUNTY

Drew C. Dingler, AIA, President
New Jersey Architect LIC.# 21GI00800

Linda M. Vivenzi, AIA, Secretary/Treasurer
New Jersey Architect LIC.# 21GI00650500

KEY PLAN:



THE DESIGN COLLABORATIVE
ARCHITECTS AND PLANNERS, P.A.
1940 ROUTE 9 NORTH
CAPE MAY COURT HOUSE, NEW JERSEY 08210
PHONE: 609 465 4111 FAX: 609 465 9588
WWW.TDCARCHITECT.COM

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SCALE:	AS NOTED	DRAWN BY:	DCD
COMM. No.	8319	CHECKED BY:	DCD
DATE:	11/1/2024		

SHEET TITLE:
ELEVATIONS AND WALL SECTIONS

SHEET No.:
A2.1

FILE G:\PROJECTS\8319-ARCHITECTURAL\A1-A8 SERIES\A2\8319 A2.1 BUILDING ELEVATIONS AND SECTIONS.DWG DATE 11/5/2026 12:24 PM USER drew dingler

STRUCTURAL NOTES

GENERAL NOTES:

1. THIS PROJECT HAS BEEN DESIGNED USING THE 2021 INTERNATIONAL BUILDING CODE NEW JERSEY EDITION.
2. WORK OF THIS CONTRACT SHALL BE COORDINATED WITH THE REQUIREMENTS OF THE GEOTECHNICAL REPORT PREPARED BY THE ATLANTIC ENGINEERING LAB DATED SEPTEMBER 28, 2023.
3. STRUCTURAL SPECIAL INSPECTIONS ARE A REQUIREMENT FOR THIS PROJECT. A QUALIFIED INDEPENDENT INSPECTION AGENCY SHALL BE SELECTED TO PERFORM THIS SERVICE. ALL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE (SEE THE FOLLOWING TABULAR REFERENCES) ARE REQUIRED AT A MINIMUM. FOR STEEL CONSTRUCTION REFER TO TABLE 1704.3, FOR CONCRETE CONSTRUCTION SEE TABLE 1704.4, FOR MASONRY CONSTRUCTION SEE TABLE 1704.5.3, FOR SOILS SEE TABLE 1704.7. SEE THE NOTES ON THIS DRAWING FOR ANY ADDITIONAL INSPECTIONS REQUIRED.
4. ALL CONTRACTORS AND SUBCONTRACTORS ARE RESPONSIBLE FOR ADHERING TO THE REQUIREMENTS AS INDICATED IN THE NOTES FOR THIS JOB. FAILURE OF THE CONTRACTOR TO READ THE STRUCTURAL NOTES DOES NOT PERMIT THE CONTRACTOR TO DEVIATE FROM THEIR REQUIREMENTS.
5. NO FIELD MODIFICATIONS TO ANY STRUCTURAL COMPONENTS SHALL BE MADE WITHOUT PRIOR APPROVAL BY THE STRUCTURAL ENGINEER. THIS INCLUDES, BUT IS NOT LIMITED TO REVISIONS DUE TO MIS-LOCATION, MISFIT, OR ANY OTHER CONSTRUCTION ERRORS.
6. ALL CONSTRUCTION AND DEMOLITION SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES INCLUDING ALL OSHA REGULATIONS.
7. CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PROTECT ALL PEOPLE WHO MAY BE ON OR NEAR THE WORK AREA, BY MAINTAINING A SAFE WORK AREA, SAFE WORKING CONDITIONS, AND LIMITING ACCESS TO THE WORK AREA.
8. CONTRACTOR IS FULLY RESPONSIBLE FOR HIS WORKERS' SAFETY, SAFETY EQUIPMENT, FIRST AID, AND EMERGENCY HANDLING PROCEDURES.
9. CONTRACTOR SHALL PERSONALLY SUPERVISE THE WORK AND SHALL BE PRESENT AT THE WORK SITE AT ALL TIMES DURING CONSTRUCTION WORK. CONTRACTOR SHALL PROVIDE ADEQUATE PERSONNEL FOR THE PROPER COORDINATION AND EXPEDITING OF THE WORK.
10. THESE DRAWINGS SHALL NOT BE SCALED FOR PURPOSES OF CONSTRUCTION.
11. TYPICAL DETAILS ARE NOT NECESSARILY REFERENCED ON EVERY DRAWING SHEET AND SHALL BE USED BY THE CONTRACTOR AS REQUIRED FOR ALL CONDITIONS WHERE APPLICABLE.
12. IN CASE OF CONFLICT BETWEEN STRUCTURAL DRAWINGS AND OTHER DRAWINGS OF THIS PROJECT, CONTRACTOR SHALL IMMEDIATELY CONTACT ARCHITECT FOR CLARIFICATION PRIOR TO START OF WORK.
13. THE CONTRACTOR IS RESPONSIBLE FOR SURVEYING AND VERIFICATION OF EXISTING CONDITIONS INCLUDING BUT NOT LIMITED TO THE LOCATION, ELEVATIONS AND DIMENSIONS OF EXISTING WALLS AND FRAMING.
14. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, INSTALLATION, AND TEMPORARY SHORING OF THE EXCAVATIONS AND BUILDING STRUCTURE AS REQUIRED DURING CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. DESIGN OF SHEETING, SHORING, SCAFFOLDING, FORM WORK, AND OTHER MEANS AND METHODS STRUCTURES SHALL BE DESIGNED BY ENGINEERS HIRED BY THE CONTRACTOR.
15. SECTIONS SHOWN ON PLANS APPLY TO SIMILAR CONDITIONS THROUGHOUT THE BUILDING.
16. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH ALL DRAWINGS FOR THE PROJECT FOR THE FOLLOWING INFORMATION.
    A. LOCATION OF ALL REQUIRED OPENINGS IN WALLS, FLOORS, ROOF, ETC. ALL OPENINGS MAY NOT BE INDICATED ON STRUCTURAL DRAWINGS.
    B. SIZE AND LOCATION OF ALL SLEEVES, INSERTS, AND DEPRESSIONS.
    C. LOCATION AND SIZE OF ALL EQUIPMENT HOUSE KEEPING PADS.

SHOP DRAWINGS AND SUBMITTALS:

1. FOR A LISTING OF SHOP DRAWINGS AND OTHER SUBMITTALS REQUIRED FOR THIS PROJECT SEE THE SHOP DRAWING AND SUBMITTAL REQUIREMENTS TABLE. CERTAIN SUBMITTALS MUST BE PREPARED UNDER THE SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF THE PROJECTS JURISDICTION. THE CONTRACTOR IS REQUIRED TO RETAIN SPECIALTY ENGINEERS AS REQUIRED TO PREPARE THESE SUBMITTALS.
2. SHOP DRAWINGS AND RELATED MATERIALS PREPARED BY SUPPLIERS AND SUBCONTRACTORS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTING TO THE ARCHITECT/STRUCTURAL ENGINEER. THE GENERAL CONTRACTOR SHALL REVIEW ALL SUBMISSIONS FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS, MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATION OF CONSTRUCTION, TECHNICAL CONTENT, COORDINATION OF TRADES, DIMENSIONAL ACCURACY, SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO, ALL OF WHICH ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THE GENERAL CONTRACTOR SHALL APPROVE AND SO STAMP EACH SUBMISSION.
3. SHOP DRAWINGS, WHERE REQUIRED, SHALL BE SUBMITTED AS FOLLOWS. PROVIDE ONE (1) ELECTRONIC PDF COPY TO THE ENGINEER FOR REVIEW. ONE (1) COPY WILL BE MARKED UP AND RETURNED FOR DISTRIBUTION AS REQUIRED BY THE CONTRACTOR. ALL SHOP DRAWINGS SHALL BE CHECKED PRIOR TO SUBMISSION. CONTRACTOR SHALL ALLOW (10) WORKING DAYS IN THE CONSTRUCTION SCHEDULE FOR SHOP DRAWING REVIEW. FAX SUBMITTALS OF SHOP DRAWINGS WILL NOT BE ACCEPTED.
4. STRUCTURAL DESIGN DRAWINGS (INCLUDING ORIGINAL CAD DRAWINGS) SHALL NOT BE USED AS THE BACKGROUNDS FOR THE PRODUCTION OF ANY SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED. THIS INCLUDES REBAR PLACEMENT DRAWINGS, FABRICATION DRAWINGS, ERECTION DRAWINGS, ERECTION DETAILS, ETC. THE CONTRACTOR SHALL PREPARE THEIR OWN SHOP DRAWINGS (INCLUDING DETAILS).
5. ANY DEVIATIONS FROM THE ORIGINAL DESIGN OR DESIGN CRITERIA AS SPECIFIED ON THE "ISSUED FOR CONSTRUCTION" DESIGN DOCUMENTS OF THE PROJECT SHALL BE NOTED (BUBBLED, NOTE, ETC.) ON THE SHOP DRAWINGS THAT ARE SUBMITTED FOR APPROVAL.
6. REVIEW OF SHOP DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR OF ANY CONTRACT REQUIREMENTS EVEN IF SUCH ITEMS ARE NOT SHOWN ON THE SHOP DRAWINGS. THE ENGINEER'S REVIEW OF SHOP DRAWINGS IS ONLY FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND PROJECT REQUIREMENTS, AND DOES NOT IMPLY APPROVAL OR VARIANCE FROM THE CONTRACT DOCUMENTS. QUANTITIES WILL NOT BE CHECKED BY THE ENGINEER.
7. ALL REVISIONS TO SHOP DRAWINGS AFTER THE FIRST SUBMISSION SHALL BE APPROPRIATELY IDENTIFIED ON SUBSEQUENT SUBMISSIONS.
8. SUBSTITUTIONS TO PRODUCTS SPECIFIED ON THE DRAWINGS IS ACCEPTABLE PROVIDED THE FOLLOWING CRITERIA ARE MET. THE CONTRACTOR SHALL SUBMIT INFORMATION ON THE PRODUCT TO BE SUBSTITUTED THAT SUBSTANTIATES ITS PERFORMANCE ON AN EQUAL OR BETTER VALUE. CONTRACTOR SHALL ALLOW A MINIMUM OF (5) WORKING DAYS IN THE CONSTRUCTION SCHEDULE FOR REVIEW OF THE SUBSTITUTED PRODUCT BY THE ENGINEER.

CONCRETE:

1. THE PROVISIONS OF ACI 318-19 HAVE BEEN UTILIZED FOR THE DESIGN OF CONCRETE ELEMENTS ON THIS PROJECT.
2. FLOOR FINISH TOLERANCES FOR THE SLAB ON GRADE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 117. FLOOR FINISH TOLERANCE SHALL BE MEASURED USING A 10 FOOT STRAIGHTEDGE ANYWHERE ON THE SLAB AND ALLOWING IT TO REST UPON TWO HIGH SPOTS WITHIN 72 HOURS AFTER SLAB PLACEMENT. THE GAP AT ANY POINT BETWEEN THE STRAIGHT EDGE AND THE FLOOR SHALL NOT EXCEED 1/4".
3. ALL CONCRETE SHALL BE NORMAL WEIGHT, READY-MIX. ALL CONCRETE MIX DESIGNS SHALL BE DESIGNED BY ENGINEERS RETAINED BY THE CONCRETE SUPPLIER ACCORDING TO THE CRITERIA CONTAINED WITHIN THESE NOTES AND AS SHOWN ON THE CONTRACT DRAWINGS. SUBMIT ALL CONCRETE MIX DESIGNS TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION. ALL SUBMITTED MIX DESIGNS SHALL INCLUDE SAMPLE CYLINDER BREAK TEST RESULTS CONFIRMING COMPRESSIVE STRENGTH OF EACH MIX DESIGN.
4. ALL CONCRETE SHALL HAVE A WATER REDUCING ADMIXTURE AS REQUIRED TO INCREASE WORKABILITY. WORKABILITY SHALL NOT BE ACHIEVED THROUGH THE ADDITION OF WATER TO THE MIX. CONCRETE SLUMP PRIOR TO ADMIXTURE ADDITION SHALL BE A MAXIMUM OF 3 INCHES. PROPORTIONS OF CONCRETE ADMIXTURES SHALL BE DETERMINED BY THE CONCRETE MIX DESIGNER.
5. DO NOT USE ADMIXTURES THAT CONTAIN CHLORIDES. FLY ASH OR OTHER POZZOLANS SHALL NOT BE USED IN ANY CONCRETE UNLESS APPROVED BY THE ENGINEER.
6. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE LATEST EDITIONS OF THE FOLLOWING ACI PUBLICATIONS ACI 301 (SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS), ACI 302.1R (GUIDE TO CONCRETE FLOOR AND SLAB CONSTRUCTION), ACI 304 (GUIDE FOR MEASURING, MIXING, TRANSPORTING AND PLACING CONCRETE), ACI 311.4 (GUIDE FOR CONCRETE INSPECTION), ACI 315 (DETAILS AND DETAILING OF CONCRETE REINFORCEMENT), ACI 318 (BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE), ACI 347R (GUIDE TO FORMWORK FOR CONCRETE), AND ACI 546R (GUIDE TO CONCRETE REPAIR). IN ADDITION, REFER TO THE CRSI - MANUAL OF STANDARD PRACTICE FOR DETAILS ON THE FABRICATION AND PLACEMENT OF CONCRETE REINFORCING.
7. PRIOR TO FABRICATION OR SHIPMENT OF MATERIAL, THE CONTRACTOR SHALL SUBMIT AND RECEIVE APPROVAL OF SHOP DRAWINGS. SHOP DRAWINGS SHALL INDICATE BENDING DIAGRAMS, SPLICING, LAPPING, SHAPES, DIMENSIONS AND DETAILS OF ALL BAR REINFORCING. THE APPROVAL OF SHOP DRAWINGS WILL BE FOR ARRANGEMENT ONLY AND SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FOR ERRORS, OMISSIONS OR THE ACCURACY OF HIS OWN DIMENSIONS. DRAWINGS AND DETAILS SHALL CONFORM WITH ACI 315. CONTRACTOR SHALL SUBMIT ALL SHOP DRAWINGS TO THE OWNER'S REPRESENTATIVE.
8. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A615 GRADE 60.
9. FOOTING, SLAB, AND WALL REINFORCEMENT NOT SHOWN ON SECTIONS AND PLANS IS THE SAME AS THAT SHOWN IN SIMILAR SECTIONS AND AT SIMILAR LOCATIONS.
10. LAP ALL BARS PER TABLE 1 LAP SPLICE LENGTHS FOR CONCRETE, CLASS B. LAP ALL WWF A MINIMUM OF 8 INCHES.
11. CONTRACTOR SHALL PROVIDE ALL BOLSTERS, CHAIRS, BAR POSITIONERS, ETC. AS REQUIRED TO SET REBAR AND SLAB WWF TO REQUIRED DIMENSIONS INDICATED ON DRAWINGS.
12. CONTROL JOINTS FOR SLABS-ON-GRADE SHALL BE SAW CUT IN ACCORDANCE WITH THE PATTERN AS INDICATED ON THE STRUCTURAL DRAWINGS. THE SPACING OF CONTROL JOINTS SHALL BE ARRANGED SUCH THAT THE AREA OF CONCRETE SLAB BETWEEN CONTROL JOINTS DOES NOT EXCEED 225 SQUARE FEET (MAXIMUM). COORDINATE WITH THE STRUCTURAL CONTRACT DRAWINGS FOR TYPICAL CONTROL JOINT DETAILS.
13. CONSTRUCTION JOINTS IN SLABS AND GRADE BEAMS SHALL BE AT MID-SPAN AND KEY JOINTED WITH REINFORCING CONTINUOUS ACROSS JOINT. COORDINATE WITH ENGINEER FOR CONSTRUCTION JOINT LOCATIONS PRIOR TO CONSTRUCTION. CONSTRUCTION JOINTS ARE TO BE LOCATED WITH RESPECT TO PARTITIONS, FLOOR FINISHES, DEPRESSIONS, ETC. AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
14. PROVIDE CONTINUOUS KEYWAYS AND DOWELS IN THE TOP OF WALL FOOTINGS SUPPORTING CONCRETE WALLS. AT CONSTRUCTION JOINTS IN CONCRETE WALLS PROVIDE KEYWAYS AND CONTINUE REINFORCING THROUGH THE JOINT.
15. REFER TO ARCHITECTURAL DRAWINGS FOR ALL WATERPROOFING DETAILS AT FOUNDATIONS, WALLS, AND SLABS.
16. AT CONCRETE WALL INTERSECTIONS PROVIDE CORNER BARS EQUAL IN SIZE AND SPACING TO TYPICAL WALL REINFORCING STEEL.
17. EPOXY COATED REINFORCING STEEL SHALL CONFORM TO ASTM A775.
18. ALL CONCRETE PLACED AT TEMPERATURES BELOW 50 DEGREES F. SHALL CONFORM TO THE REQUIREMENTS OF ACI 306R "GUIDE TO COLD WEATHER CONCRETING". ALL CONCRETE PLACED IN HOT WEATHER SHALL CONFORM TO THE REQUIREMENTS OF ACI 305R " GUIDE TO HOT WEATHER CONCRETING".
19. THE CONTRACTOR SHALL COORDINATE THE LOCATION OF ALL SLEEVES, INSERTS, ANCHOR BOLTS, AND OTHER EMBEDDED ITEMS AS REQUIRED BY OTHER TRADES.
20. ALL CONCRETE SHALL BE PROPERLY CONSOLIDATED THROUGH THE USE OF VIBRATORS. VIBRATORS SHALL NOT BE USED TO TRANSPORT CONCRETE ALONG FORMWORK.
21. CONTRACTOR SHALL FOLLOW THE GUIDELINES IN ACI 303 - GUIDE TO CAST-IN-PLACE ARCHITECTURAL CONCRETE PRACTICE TO ACHIEVE SPECIFIED SURFACE FINISHES OF EXPOSED CONCRETE. COORDINATE WITH ARCHITECTURAL DRAWINGS FOR ADDITIONAL REQUIREMENTS ON FINISH.
22. UNLESS OTHERWISE SPECIFIED, A TESTING AGENCY SHALL BE EMPLOYED FOR EVALUATION AND QUALITY CONTROL OF CONCRETE PLACED. THE TESTING AGENCY PERFORMING ACCEPTANCE TESTING SHALL COMPLY WITH ASTM C1077. CONCRETE SHALL BE TESTED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318. FREQUENCY OF CONCRETE TESTING SHALL MEET THE REQUIREMENTS OF ACI 318 AT A MINIMUM UNLESS REQUIRED OTHERWISE BY THE APPLICABLE BUILDING CODE.

CONCRETE MIX DESIGN AND DURABILITY REQUIREMENTS PER ACI 318 BUILDING CODE

LOCATION	DENSITY NW = 145 pcf LW = 115 pcf	FREEZE/THAW SEVERITY	SULFATE SEVERITY	PERMEABILITY	CORROSION PROTECTION OF REINF	f 'c (psi)	AIR CONTENT	w / c RATIO (max)
FOOTINGS	NW	F0	S0	P0	C1	4,000	----	0.50
EXTERIOR WALLS / PIERS	NW	F1	S0	P0	C1	4,500	6 %	0.45
EXTERIOR SLABS	NW	F3	S0	P0	C2	5,000	6 %	0.40

CONCRETE MIX DESIGN & DURABILITY NOTES:

1. CONCRETE MIX SHALL BE DESIGNED BY THE CONCRETE SUPPLIER USING THE INFORMATION CONTAINED IN THIS SCHEDULE.
2. REFER TO CHAPTER 19 OF THE ACI-318 BUILDING CODE FOR ADDITIONAL INFORMATION NOT PROVIDED OR NOTED IN THIS SCHEDULE.
3. TOTAL AIR CONTENT LISTED IN THIS SCHEDULE IS BASED ON A MAXIMUM AGGREGATE SIZE OF 3/4" AND SHALL BE ADJUSTED BY THE CONCRETE MIX DESIGNER AS REQUIRED FOR DIFFERENT AGGREGATE SIZES PER ACI-318.
4. REFER TO THE CONCRETE NOTES ON THE LEAD SHEET FOR THIS PROJECT FOR ADDITIONAL REQUIREMENTS.
5. FLY ASH OR OTHER POZZOLANS SHALL NOT BE UTILIZED IN ANY CONCRETE MIX UNLESS APPROVED BY THE ENGINEER. THE QUANTITY OF POZZOLANS USED IN CONCRETE SUBJECT TO EXPOSURE CLASS F3 SHALL NOT EXCEED THE LIMITS SET FORTH IN ACI-318.
6. THE MINIMUM AMOUNT OF CEMENT TO BE USED IN THE CONCRETE MIX DESIGN IS AS OF FOLLOWS:
   4,000 PSI CONCRETE: 600 LBS OF CEMENT PER CUBIC YARD
   4,500 PSI CONCRETE: 650 LBS OF CEMENT PER CUBIC YARD
   5,000 PSI CONCRETE: 700 LBS OF CEMENT PER CUBIC YARD
7. FOR ALL SLABS ADD PENETRON ADMIX SB, DOSAGE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
8. COAT ALL CONCRETE SLABS WITH PENETRON CRYSTALLINE WATERPROOFING COATING IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

FOUNDATIONS:

1. SPECIAL INSPECTIONS FOR EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT, AND LOAD BEARING REQUIREMENTS SHALL BE IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE AND TABLE 1704.7.
2. BOTTOM OF ALL FOOTINGS HAVE BEEN DESIGNED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 3,000 PSF.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING THE APPROPRIATE AUTHORITIES TO LOCATE ALL POTENTIALLY BURIED UTILITIES WITHIN THE PROPOSED PROJECT SITE BUILDING FOOTPRINT PRIOR TO COMMENCING EXCAVATION FOR NEW BUILDING FOUNDATIONS.
4. EXISTING FOUNDATIONS, SLABS, PAVEMENTS, UNDERGROUND UTILITIES, AND OTHER BELOW GRADE STRUCTURES SHALL BE REMOVED FROM THE PROPOSED PROJECT SITE BUILDING FOOTPRINT. REMOVE SURFACE VEGETATION, TOPSOIL, ROOT SYSTEMS, ORGANIC MATERIAL, EXISTING FILL, AND SOFT UNSUITABLE MATERIAL FROM THE BUILDING AREA.
5. CONTRACTOR SHALL EXERCISE EXTREME CAUTION DURING NEW FOUNDATION CONSTRUCTION ACTIVITIES ADJACENT TO EXISTING BUILDING FOUNDATIONS THAT ARE TO REMAIN (EVEN IF LOCATED ON AN ADJACENT PROPERTY). SINCE DRAWINGS FOR EXISTING CONSTRUCTION ARE NOT ALWAYS AVAILABLE DURING DESIGN, CERTAIN ASSUMPTIONS MAY BE MADE REGARDING EXISTING FOUNDATIONS BASED ON TYPICAL CONSTRUCTION PRACTICES. THESE ASSUMPTIONS TYPICALLY REQUIRE CONTRACTOR FIELD VERIFICATION PRIOR TO CONSTRUCTION OF THE NEW STRUCTURES. IN ANY EVENT, THE ENGINEER MUST BE NOTIFIED IMMEDIATELY IF EXISTING SITE OR FOUNDATION CONDITIONS DIFFER FROM THOSE SHOWN OR ASSUMED ON THE CONTRACT DRAWINGS. IN NO INSTANCE SHALL EXISTING BUILDING FOUNDATIONS BE UNDERMINED TO INSTALL NEW FOUNDATIONS. IF NEW BOTTOM OF FOOTING ELEVATIONS ARE LOWER THAN ADJACENT EXISTING BOTTOM OF FOOTING ELEVATIONS THE ENGINEER MUST BE NOTIFIED IMMEDIATELY TO PROVIDE ADDITIONAL DETAILS AS REQUIRED TO CONSTRUCT THE NEW FOUNDATIONS AT THE LOWER LEVEL.
6. THE CONTRACTOR MUST PROVIDE SURFACE DRAINAGE AND PUMPS TO PROTECT ALL EXCAVATION FROM FLOODING OR GROUND WATER INFILTRATION. FLOODING OF ANY EXCAVATION AFTER APPROVAL OF THE SUBGRADE WILL BE CAUSE FOR COMPLETE RE-PREPARATION OF THE SUBGRADE.
7. BOTTOM OF ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE PLACING ANY CONCRETE. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN A SOIL BEARING PRESSURE OF 3,000 PSF BELOW ALL FOOTINGS.
8. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF THREE FEET (3'-0") BELOW EXTERIOR FINISH GRADE.
9. STANDARD PROCEDURES FOR FROST PROTECTION OF FOUNDATIONS AND EXCAVATIONS SHALL BE EMPLOYED FOR WINTER CONSTRUCTION. BACKFILLING OF EXCAVATIONS SHALL BE DONE AS SOON AS POSSIBLE TO PROTECT FOUNDATIONS FROM FROST.
10. DUE TO THE PRESENCE OF UNSUITABLE BEARING MATERIALS, OVER EXCAVATION WILL BE REQUIRED IN SOME AREAS TO PROVIDE THE SAFE ALLOWABLE BEARING PRESSURE LISTED ABOVE. CONTRACTOR SHALL FOLLOW THE REQUIREMENTS OF THE REFERENCED GEOTECHNICAL REPORT LISTED ABOVE.
11. RETAINING WALLS SHALL HAVE CONTROL JOINTS AT 30 FEET MAXIMUM ON CENTERS UNLESS NOTED OTHERWISE.
12. UNLESS OTHERWISE DICTATED BY THE GEOTECHNICAL ENGINEER, ALL FILL AND BACKFILL SHALL BE COMPACTED IN 8 INCH MAXIMUM LIFTS TO NOT LESS THAN 95% OF THE MAXIMUM DENSITY IN ACCORDANCE WITH ASTM D1557.
13. HORIZONTAL REINFORCING BARS IN FOUNDATIONS SHALL BE CONTINUOUS. PROVIDE CORNER BARS AT ALL CORNERS AND INTERSECTIONS.
14. FOUNDATION PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE ENGINEER. PENETRATIONS SHALL BE THROUGH THE FOUNDATION STEM WALL OR 6" CLEAR BELOW FOOTINGS.
15. ALL CMU WALL FOUNDATION WALLS SHALL BE FILLED SOLID WITH 3,000 PSI GROUT BELOW FINISHED GRADE ELEVATION UNLESS NOTED OTHERWISE ON PLANS.

MASONRY:

1. ALL IVANY BLOCK SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,800 PSI ON THE NET AREA OF INDIVIDUAL UNITS. ALL CMU SHALL BE ERECTED IN A RUNNING BOND PATTERN AND SHALL BE LAID IN A FULL BED OF MORTAR.
2. ALL MORTAR SHALL BE PORTLAND CEMENT/LIME CONFORMING TO ASTM C270. ALL IVANY BLOCK SHALL BE LAID USING ASTM C270 TYPE M MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS. DO NOT USE ADMIXTURES THAT CONTAIN CHLORIDES.
3. GROUT SHALL BE A HIGH SLUMP MIX IN ACCORDANCE WITH ASTM SPECIFICATION C476 HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS. GROUT MUST HAVE A SLUMP OF 8 TO 11 INCHES. DO NOT USE ADMIXTURES THAT CONTAIN CHLORIDES. DO NOT SUBSTITUTE MORTAR FOR GROUT. CONSOLIDATION OF GROUT IN BLOCK CORES SHOULD BE ACHIEVED WITH A LOW VELOCITY MECHANICAL VIBRATOR WHICH HAS A 3/4 INCH HEAD. THE VIBRATOR IS NORMALLY ACTIVATED FOR ONE OR TWO SECONDS IN EACH GROUTED CORE OF HOLLOW UNIT MASONRY.
4. LAID UP MASONRY STRENGTH f'm FOR THE COMPOSITE OF CMU, MORTAR AND GROUT FOR ALL STANDARD MASONRY WALLS SHALL BE 2,800 PSI FOR IVANY BLOCK AND PIERS.
5. ALL CONCRETE MASONRY SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES ACI 530/ASCE 5/TMS 402" AND "SPECIFICATIONS FOR MASONRY STRUCTURES ACI 530.1/ASCE 6/TMS 602".
6. PERFORM INSPECTIONS OF MASONRY CONSTRUCTION IN ACCORDANCE WITH THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE.
7. ALL REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60.
8. PROVIDE HOT-DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (MIN. 9 GAGE COLD DRAWN GALVANIZED STEEL WIRE) IN ACCORDANCE WITH ASTM A1064 AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS. SPACE HORIZONTAL JOINT REINFORCEMENT AT 8" INCHES ON CENTER IN ALL PARAPETS. USE SHOP FABRICATED SPECIAL PIECES AT ALL CORNERS AND TEES.
9. ALL CMU CORES WHICH CONTAIN VERTICAL REINFORCING BARS SHALL BE GROUTED SOLID. ALL CMU PIERS SHOWN ON PLAN SHALL BE COMPLETELY GROUTED SOLID IN ALL CORES. FILLING CORES WITH MORTAR AS WORK PROGRESSES IS NOT ACCEPTABLE.
10. MINIMUM LENGTH OF LAP SPLICES FOR VERTICAL REINFORCING SHALL NOT BE LESS THAN THE VALUE PROVIDED IN THE CMU REINFORCING LAP AND SPLICE LENGTH TABLE.
11. VERTICAL REINFORCING BARS SHALL BE SECURED AGAINST DISPLACEMENT PRIOR TO GROUTING BY GALVANIZED BAR POSITIONERS SPACED AT INTERVALS NOT TO EXCEED 112 BAR DIAMETERS. PROVIDE A MINIMUM OF TWO POSITIONERS PER INDIVIDUAL REINFORCING BAR.
12. SUBMIT CHECKED SHOP DRAWINGS WHICH DETAIL THE LOCATION OF VERTICAL REINFORCEMENT PLACEMENT FOR ALL WALLS AND PIERS. THESE DRAWINGS SHALL CONTAIN THE FOLLOWING INFORMATION: WALL ELEVATIONS SHOWING LOCATION AND SIZE OF REINFORCING STEEL, LOCATION OF REINFORCING STEEL SPLICES, AND LOCATION OF BAR POSITIONERS. WALL DETAILS WHICH INDICATE WHERE REINFORCING STEEL IS LOCATED IN EACH CORE. (I.E. CENTER OF CORE, NEAR INTERIOR FACE OF WALL/PIER, NEAR EXTERIOR FACE OF WALL/PIER). DETAIL OF BAR POSITIONERS. PIER DETAILS AND ELEVATIONS SHOWING THE LOCATION AND SIZE OF REINFORCING STEEL, LOCATION OF REINFORCING STEEL SPLICES, AND LOCATION OF BAR POSITIONERS.
13. MASON TO DESIGN, FURNISH AND INSTALL ALL REQUIRED SHORING FOR ERECTION OF THE CMU MASONRY WALLS AND PIERS.
14. PROVIDE CONTROL JOINTS IN MASONRY WALLS AT A MAXIMUM SPACING OF 30 FEET ON CENTER UNLESS OTHERWISE NOTED ON THE CONTRACT DRAWINGS. CONTROL JOINTS SHALL BE LOCATED AT ALL CHANGES IN MASONRY WALL HEIGHT AND AT ALL CHANGES IN MASONRY WALL THICKNESS. COORDINATE WITH THE ARCHITECT FOR ALL MASONRY CONTROL JOINT LOCATIONS.
15. PROVIDE CONTINUOUS KNOCK-OUT CMU BOND BEAMS IN ALL WALLS AS PER THE FOLLOWING CRITERIA UNLESS NOTED OTHERWISE ON THE CONTRACT DRAWINGS. 8" KNOCK-OUT CMU BOND BEAMS SHALL BE REINFORCED WITH (1) #5 BAR CONTINUOUS AND 12" KNOCK-OUT CMU BOND BEAMS SHALL BE REINFORCED WITH (2) #5 BARS CONTINUOUS. ALL BOND BEAMS SHALL BE FILLED SOLID WITH 3,000 PSI GROUT.
    A. PROVIDE CONTINUOUS KNOCK-OUT BOND BEAMS AT THE TOP OF ALL CMU WALLS.
    B. PROVIDE CONTINUOUS KNOCK-OUT BOND BEAMS FOR ELEVATOR AND STAIR SHAFT WALLS AT EACH FLOOR.

POST-INSTALLED ADHESIVE ANCHORS & REINFORCING:

1. THE ADHESIVE ANCHOR SYSTEM USED FOR POST-INSTALLED ANCHORAGE TO CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF THE MOST RECENTLY PUBLISHED ACI 355.4, ACCEPTANCE CRITERIA FOR QUALIFICATION OF POST-INSTALLED ADHESIVE ANCHORS IN CONCRETE AND COMMENTARY.
2. THE ADHESIVE ANCHORS SHALL BE SUPPLIED AS AN ENTIRE SYSTEM. THE SYSTEM SHALL INCLUDE, BUT IS NOT LIMITED TO, THE NEW ADHESIVE CARTRIDGE, A CLEAN MIXING NOZZLE, EXTENSION TUBE, A DISPENSING GUN, AND ALL MANUFACTURER RECOMMENDED SUPPLIES FOR PROPERLY CLEANING THE DRILLED HOLE.
3. EYEBOLTS, THREADED STUDS, INTERNAL THREADED PARTS TO BE USED IN ADHESIVE ANCHOR ASSEMBLIES SHALL CONFORM TO ASTM A36, A193 (GRADE B7), A307, B348 (BD), OR F1554. STAINLESS STEEL ANCHOR RODS SHALL BE AISI TYPE 304 OR TYPE 316. THREADS SHALL BE UNC COARSE THREADS, UNLESS NOTED OTHERWISE. COMPATIBLE NUTS AND WASHERS SHALL BE FURNISHED WITH THE ALL-THREAD ROD AND CONSIDERED PART OF THE ASSEMBLY. THE COST OF THE HARDWARE SHALL BE CONSIDERED INCIDENTAL TO THE INSTALLED ADHESIVE ANCHOR ASSEMBLY.
4. NUTS, WASHERS, AND OTHER HARDWARE USED WITH AN ALL-THREADED BAR ADHESIVE ANCHOR SYSTEM SHALL HAVE A MATERIAL OR AN ALLOY DESIGNATION THAT MATCHES THE ALL-THREAD MATERIAL / ALLOY. GALVANIZED ASSEMBLIES SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A153 CLASS C. ELECTROPLATE GALVANIZING IS NOT ACCEPTABLE. DISSIMILAR METAL ASSEMBLIES SHALL BE SEPARATED BY NYLON, EPDM, OR OTHER APPROVED NON-METALLIC WASHERS.
5. REINFORCING BARS TO BE USED IN ADHESIVE ANCHORS ASSEMBLIES SHALL CONFORM TO ASTM A615.
6. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'c) OF 2,500 PSI AT THE TIME OF ADHESIVE ANCHOR INSTALLATION.
7. CONCRETE AT TIME OF ADHESIVE ANCHOR INSTALLATION SHALL HAVE A MINIMUM AGE OF 21 DAYS.
8. CONCRETE TEMPERATURE AT THE TIME OF ADHESIVE ANCHOR INSTALLATION SHALL BE AT LEAST 50 DEGREES F.
9. EMBEDMENT DEPTH AND ANCHOR PROJECTION (STICK-OUT) FROM THE CONCRETE SURFACE SHALL BE AS SHOWN ON THE DRAWING OR DETAIL FOR THE PARTICULAR ANCHOR OR GROUP OF ANCHORS BEING INSTALLED. ABSENT ANY INFORMATION, THE MINIMUM EMBEDMENT DEPTH SHALL BE 10 TIMES THE ANCHOR DIAMETER IN INCHES AND MINIMUM STICK-OUT SHALL BE AS REQUIRED TO MAKE THE CONNECTION.
10. ADHESIVES SHALL BE STORED AND INSTALLED AT THE SERVICE TEMPERATURE RANGES RECOMMENDED BY THE MANUFACTURER.
11. ADHESIVE ANCHORS SHALL BE INSTALLED BY QUALIFIED PERSONNEL TRAINED TO INSTALL ADHESIVE ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS OF THE MANUFACTURER AND THE CONTRACT DOCUMENTS. POST-INSTALLED ADHESIVE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.
12. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. THESE ANCHORS ARE DESIGNATED WITH A (CERT) AFTER THE ANCHOR CALL-OUT. NOTE: SOME DOWNHAND INSTALLATIONS SHOWN ON THESE DRAWINGS SUPPORT SUSTAINED TENSION LOADS AND ARE SO DESIGNATED WITH A (CERT) AFTER THE ANCHOR CALL-OUT.
13. THE INSTALLER'S QUALIFICATIONS SHALL BE SUBMITTED AND APPROVED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
14. THE CONTRACTOR SHALL PROVIDE ALL EQUIPMENT REQUIRED TO INSTALL THE ADHESIVE ANCHOR INCLUDING, BUT NOT LIMITED TO, DRILLS, SETTING TOOLS, CLEAN-OUT BRUSHES, BLOW OUT BULBS, OIL-FREE COMPRESSED AIR, SHOP VACUUMS, WRENCHES, ETC.
15. ANCHORS SHALL BE INSTALLED IN HOLES DRILLED WITH A ROTARY IMPACT HAMMER DRILL OR ROCK DRILL.
16. ANCHOR HOLES SHALL BE THOROUGHLY CLEANED PRIOR TO ADHESIVE INJECTION, AS REQUIRED BY THE MANUFACTURERS PRINTED INSTALLATION INSTRUCTIONS.
17. ANCHORS TO BE INSTALLED IN THE ADHESIVE SHALL BE CLEAN, OIL-FREE, AND FREE OF LOOSE RUST, PAINT, OR OTHER COATINGS.
18. INSTALLED ADHESIVE ANCHORS SHALL BE SECURELY FIXED IN-PLACE TO PREVENT DISPLACEMENT WHILE THE ADHESIVE CURES. UNLESS SHOWN OTHERWISE ON THE DRAWINGS, ANCHORS SHALL BE INSTALLED PERPENDICULAR TO THE CONCRETE SURFACE. ANCHORS DISPLACED BEFORE FULL ADHESIVE CURE SHALL BE CONSIDERED DAMAGED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
19. ADHESIVE ANCHORS INTO CONCRETE SUBSTRATE APPLICATIONS SHALL USE THE HILTI HIT HY-200 SYSTEM.
20. ADHESIVE ANCHORS INTO SOLID GROUTED CMU SUBSTRATE APPLICATIONS SHALL USE THE HILTI HY-270 SYSTEM.
21. ADHESIVE ANCHORS INTO HOLLOW CMU SUBSTRATE APPLICATIONS SHALL USE THE HILTI HIT HY-270 SYSTEM.
22. ALL HOLES IN STEEL MEMBERS TO RECEIVE POST-INSTALLED ADHESIVE OR EXPANSION ANCHORS SHALL BE STANDARD SIZE BASED ON THE ANCHOR DIAMETER (UNLESS NOTED OTHERWISE). OVERSIZED OR SLOTTED HOLES IN THE DIRECTION OF FORCE APPLICATION ARE NOT PERMITTED.

TABLE 1
LAP SPLICE LENGTHS FOR CONCRETE

CLASS B SPLICE LENGTHS (INCHES)

BAR SIZE	3,000 PSI CONCRETE TENSION TYP	3,000 PSI TENSION TOP	3,000 PSI COMP	4,000 PSI CONCRETE TENSION TYP	4,000 PSI TENSION TOP	4,000 PSI COMP	5,000 PSI CONCRETE TENSION TYP	5,000 PSI TENSION TOP	5,000 PSI COMP	6,000 PSI CONCRETE TENSION TYP	6,000 PSI TENSION TOP	6,000 PSI COMP
#3	21	27	12	18	24	12	17	22	12	17	20	12
#4	29	38	15	25	32	15	22	29	15	20	26	15
#5	37	47	19	31	40	19	28	36	19	25	33	19
#6	43	56	23	38	48	23	34	43	23	31	39	23
#7	62	82	27	54	70	27	49	63	27	45	58	27
#8	72	92	30	62	80	30	56	72	30	51	66	30
#9	81	104	34	70	91	34	63	81	34	57	74	34
#10	90	116	39	79	102	39	71	92	39	64	84	39

TABLE 2
STRAIGHT BAR DEVELOPMENT LENGTHS

CLASS A SPLICE LENGTHS (INCHES)

BAR SIZE	3,000 PSI CONCRETE TENSION TYP	3,000 PSI TENSION TOP	3,000 PSI COMP	4,000 PSI CONCRETE TENSION TYP	4,000 PSI TENSION TOP	4,000 PSI COMP	5,000 PSI CONCRETE TENSION TYP	5,000 PSI TENSION TOP	5,000 PSI COMP	6,000 PSI CONCRETE TENSION TYP	6,000 PSI TENSION TOP	6,000 PSI COMP
#3	16	21	9	14	18	8	13	17	8	12	16	8
#4	22	29	11	19	25	9	17	22	9	15	20	9
#5	28	36	14	24	31	12	21	29	12	19	25	12
#6	33	43	17	29	37	14	26	33	14	24	30	14
#7	48	63	19	42	54	17	38	49	16	34	45	16
#8	55	71	22	48	62	19	43	56	18	39	51	18
#9	62	80	25	54	70	21	48	63	20	44	57	20
#10	69	89	28	61	79	24	54	71	23	50	64	23

NOTES (TABLES 1, 2, AND 3):

1. DEVELOPMENT AND LAP SPLICE LENGTHS SHOWN ARE FOR UNCOATED ASTM A615 GRADE 60 (F_y = 60,000 PSI) DEFORMED BAR REINFORCING.
2. DEVELOPMENT LENGTHS ARE BASED ON NORMAL WEIGHT CONCRETE. FOR LIGHT WEIGHT CONCRETE MULTIPLY THE LENGTHS SHOWN BY 1.333.
3. MINIMUM BAR CLEAR COVER SHALL BE 1 BAR DIAMETER. MINIMUM BAR CLEAR SPACING IS 1 BAR DIAMETER IN BEAMS AND COLUMNS AND 2 BAR DIAMETERS IN OTHER CONCRETE ELEMENTS. MULTIPLY THE DEVELOPMENT LENGTH SHOWN BY 1.5 FOR REINFORCING WITH COVER AND SPACING LESS THAN DESCRIBED ABOVE.
4. USE THE LAP SPLICE LENGTHS IN THE "TOP" COLUMN FOR HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE BELOW THE BAR.

TABLE 3
HOOKED BAR TENSION DEVELOPMENT LENGTHS

LENGTHS (INCHES)

BAR SIZE	3,000 PSI CONCRETE	4,000 PSI CONCRETE	5,000 PSI CONCRETE	6,000 PSI CONCRETE
#3	9	8	7	6
#4	11	10	9	8
#5	14	12	11	10
#6	17	15	13	12
#7	19	17	15	14
#8	22	19	17	16
#9	25	22	20	18
#10	28	24	22	20

SPECIAL INSPECTION AND TESTING (IBC 2021 CHAPTER 17)

1. ALL TESTS AND INSPECTIONS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AND INSPECTION AGENCY. THE SPECIAL INSPECTOR FROM THIS TESTING AGENCY SHALL OBSERVE THE WORK FOR CONFORMANCE TO THE DESIGN DRAWINGS AND SPECIFICATIONS. THE SPECIAL INSPECTOR SHALL BE TRAINED/CERTIFIED TO PERFORM THE REQUIRED SPECIAL INSPECTIONS. THE SPECIAL INSPECTOR SHALL SUBMIT WRITTEN DOCUMENTATION OF CERTIFICATIONS FOR RECORD PRIOR TO CONSTRUCTION.
2. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE ENGINEER OR ARCHITECT OF RECORD, AND ALL OTHER DESIGNATED INDIVIDUALS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF NOT CORRECTED, TO THE PROPER DESIGN AUTHORITY AND TO THE BUILDING OFFICIAL.
3. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS, SPECIFICATIONS, SOILS REPORT AND APPLICABLE WORKMANSHIP PROVISIONS OF THE INTERNATIONAL BUILDING CODE.
4. STRUCTURAL OBSERVATIONS BY THE STRUCTURAL ENGINEER SHALL NOT BE CONSIDERED A SPECIAL INSPECTION.
5. THE FOLLOWING ITEMS MARKED "X" REQUIRE SPECIAL INSPECTIONS: (REFER TO IBC 2021 CHAPTER 17 FOR ADDITIONAL INFORMATION)

	VERIFICATION AND INSPECTION	INSPECTION REQUIRED CONTINUOUS	INSPECTION REQUIRED PERIODIC
1705.3 - CONCRETE CONSTRUCTION			
1.	Inspection of reinforcement and verification of placement		X
2.	Inspection of reinforcing bar welding (in accordance with AWS D1.4):		
a.	Verification of weldability of reinforcing bars other than ASTM A706		X
b.	Inspection of single-pass fillet welds, maximum 5/16"		X
c.	Inspection of all other welds	X	
3.	Inspection of anchors cast in concrete		X
4.	Inspection of anchors post-installed in hardened concrete members:		
a.	Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads	X	
b.	Mechanical anchors and adhesive anchors not defined in 4.a		X
5.	Verification of required design mix		X
6.	Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete	X	
7.	Inspection of concrete and shotcrete placement for proper application techniques	X	
8.	Verification of maintenance of specified curing temperature and techniques		X
9.	Verification of in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs		X
10.	Inspection of formwork for shape, location, and dimensions of the concrete member being formed		X

	VERIFICATION AND INSPECTION	INSPECTION REQUIRED CONTINUOUS	INSPECTION REQUIRED PERIODIC
1705.4 - MASONRY CONSTRUCTION			
	Special inspection for masonry construction shall be in accordance with TMS 402/ACI 530/ASCE 5 and TMS 602/ACI 530.1/ASCE 6 quality assurance program requirements. At a minimum, the following inspections are required.		
1.	Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.		X
2.	Verification of f'm and f'_{AAC} prior to construction.		X
3.	Verification of slump flow and VSI as delivered to the site for self-consolidating grout.	X	
4.	As masonry construction begins the following shall be verified:		
a.	Proportions of site prepared mortar		
b.	Grade, type and size of reinforcement, connectors, and anchor bolts		X
c.	Properties of thin-bed mortar for AAC masonry for the first 5,000 square feet	X	
d.	Properties of thin-bed mortar for AAC masonry after the first 5,000 square feet		X
e.	Sample panel construction		X
5.	Prior to grouting, the following shall be verified to ensure compliance:		
a.	Grout space		X
b.	Placement of reinforcement, connectors, and anchor bolts		X
6.	During construction the inspection program shall verify:		
a.	Materials and procedures with the approved submittals		X
b.	Placement of masonry units and mortar joint construction		X
c.	Size and location of structural members		X
d.	Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction		X
e.	Welding of reinforcement	X	
f.	Preparation, construction, and protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F)		X
g.	Placement of grout is in compliance	X	
h.	Placement of AAC masonry units and construction of thin-bed mortar joints for the first 5,000 square feet	X	
i.	Placement of AAC masonry units and construction of thin-bed mortar joints after the first 5,000 square feet		X
7.	Observe preparation of grout specimens, mortar specimens, and/or prisms		X

	VERIFICATION AND INSPECTION	INSPECTION REQUIRED CONTINUOUS	INSPECTION REQUIRED PERIODIC
1705.5 - WOOD CONSTRUCTION			
	Special inspection for wood construction shall be in accordance with IBC Section 1705.5. At a minimum, the following inspections are required:		
1.	Verify, size, species and grade of lumber/sheathing complies with construction documents.		X
2.	Verify nailing of diaphragm floor and roof sheathing and shear walls complies with construction documents.		X
3.	Verify wood shear wall hold downs conform to construction documents.		X
4.	Verify attachment of shear wall sill plates at foundations and floors.		X

	VERIFICATION AND INSPECTION	INSPECTION REQUIRED CONTINUOUS	INSPECTION REQUIRED PERIODIC
1705.6 - SOILS			
1.	Verification of materials below shallow foundations are adequate to achieve the design bearing capacity		X
2.	Verification that excavations are extended to proper depth and have reached proper material		X
3.	Perform classification and testing of compacted fill materials.		X
4.	Verification of use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill	X	
5.	Prior to placement of compacted fill, inspection of subgrade and verify that site has been prepared properly		X

DELEGATED DESIGN

1	Temporary Shoring of Excavations & Building Structure During Construction. Other Contractor Means & Methods Components (e.g. Scaffolding, Fall Protection, etc.)
2	Wood Pole Barn Building

DELEGATED DESIGN SCHEDULE NOTES:

1. THE ITEMS LISTED IN THIS SCHEDULE HAVE NOT BEEN DESIGNED BY THE STRUCTURAL ENGINEER OF RECORD FOR THIS PROJECT. A SPECIALTY ENGINEER SHALL BE RETAINED BY THE CONTRACTOR TO PERFORM THE REQUIRED DESIGNS.
2. THE SPECIALTY ENGINEER SHALL BE A LICENSED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT.
3. CALCULATIONS AND/OR SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD PRIOR TO CONSTRUCTION.

SHOP DRAWING SUBMITTAL REQUIREMENTS

			Submittal Required (Yes)	Signed & Sealed (Yes)
Section 3 - Concrete				
	1	Concrete Mix Design	X	
	2	Concrete Reinforcing Shop Drawings	X	
Section 4 - Masonry				
	1	Masonry Reinforcing Shop Drawings	X	
	2	Masonry Materials; Grout, Mortar, CMU Block & Bond Beams	X	

CMU REINFORCING LAP SPLICE LENGTHS

LENGTHS (INCHES)

BAR SIZE	BAR POSITION CENTER	BAR POSITION OFFSET	STANDARD HOOK EMBEDMENT (SEE NOTE 7)
#3	12"	15"	6"
#4	20"	26"	8"
#5	23"	40"	10"
#6	43"	54"	12"
#7	60"	63"	14"
#8	72"	82"	16"

1. TABLE BASED ON f'm = 1,500 PSI (MINIMUM) AND GRADE 60 REINFORCING.
2. THIS TABLE DOES NOT APPLY TO REINFORCING WITH EPOXY COATINGS.
3. LAP AND SPLICE LENGTHS SHOWN APPLY TO BARS IN TENSION AND COMPRESSION.
4. BARS IN OFFSET CONDITION SHALL HAVE A MINIMUM OF 2" CLEAR COVER FROM OUTSIDE FACE OF CMU TO FACE OF BAR.
5. MINIMUM LENGTH OF STANDARD HOOK = 12 BAR DIAMETERS.
6. AT CONTRACTORS OPTION, PROVIDE MECHANICAL SPLICES OF REINFORCING BARS. MECHANICAL SPLICE SHALL DEVELOP AT LEAST 125% OF THE SPECIFIED BAR YIELD STRENGTH.
7. STANDARD HOOK EMBEDMENT IS FOR HOOK EMBEDDED INTO ADJOINING MASONRY CONSTRUCTION. FOR HOOKED LENGTHS REQUIRED INTO CONCRETE SEE TABLE 3.

DATE	No.	REVISION
10/30/26	1	RELEASE FOR BID

BLOCK NO. 4704 LRHSD PROJECT NO. REF21-43-8319, 2-010-24 LOT NO. 3

PROPOSED NEW MAINTENANCE FACILITY

SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING

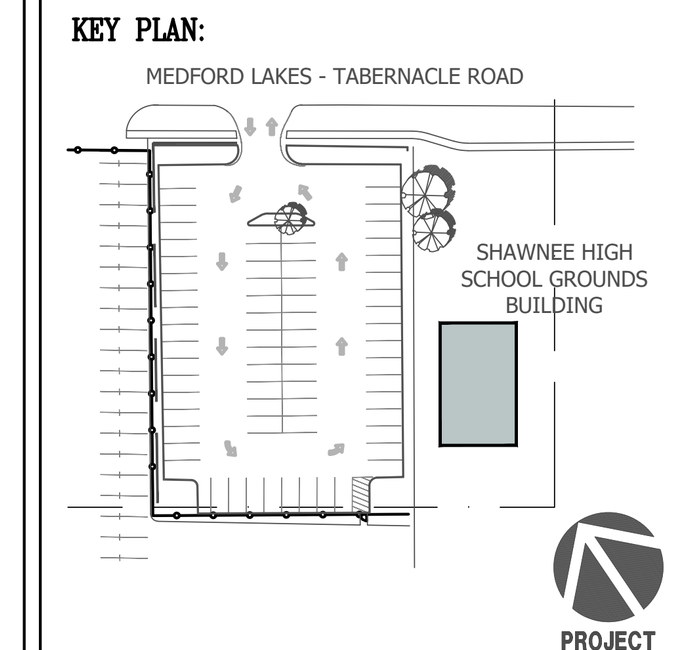
580 TABERNACLE ROAD, MEDFORD, NEW JERSEY 08055

MEDFORD TOWNSHIP BURLINGTON COUNTY

Linda M. Viventi, AIA, Secretary/Treasurer
New Jersey Architect LIC.# 21AI01090200; P.P. LIC.# 33LI00653000

Drew C. Dingler, AIA, President
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KEY PLAN:



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PHONE: 609 465 4111 FAX: 609 465 9358
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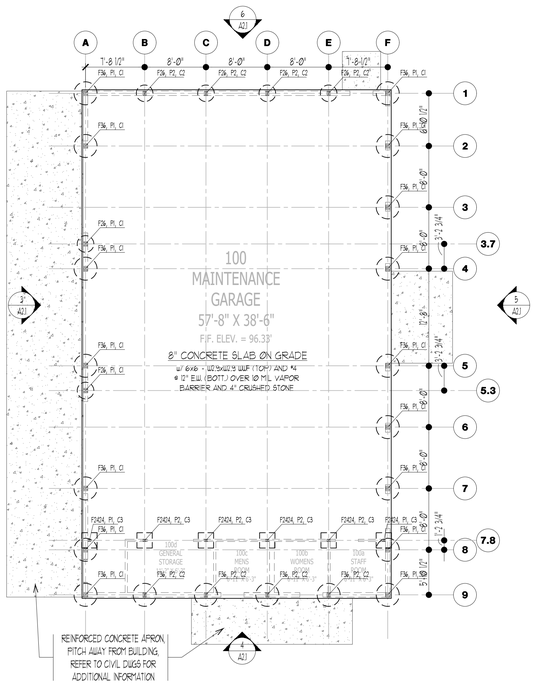
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SCALE:	AS NOTED	DRAWN BY: DCD
COMM. No.	8319	CHECKED BY: DCD
DATE:	11/1/2024	

SHEET TITLE: STRUCTURAL NOTES

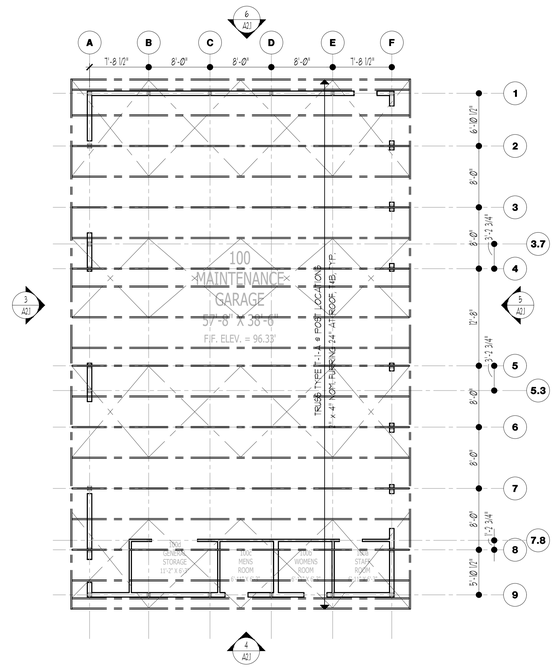
SHEET No.: S1.0

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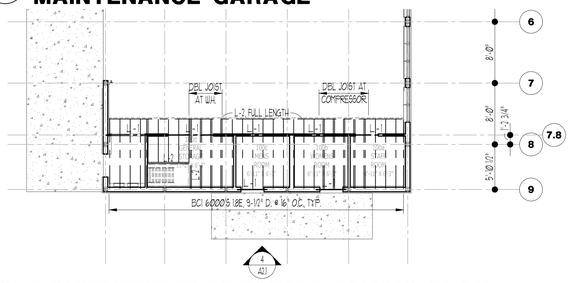
1 S1.1 FOUNDATION PLAN MAINTENANCE GARAGE

1/8"=1'-0"



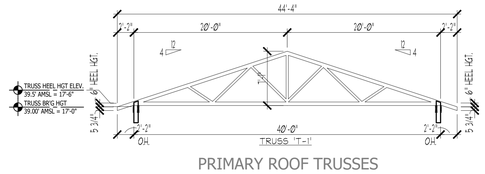
2 S1.1 ROOF FRAMING PLAN MAINTENANCE GARAGE

1/8"=1'-0"



1a S1.1 EQUIPMENT PLATFORM FRAMING PLAN MAINTENANCE GARAGE

1/8"=1'-0"



PRIMARY ROOF TRUSSES

2 S1.1 TRUSS TYPES MAINTENANCE GARAGE

1/8"=1'-0"

NOTE: TRUSSES ARE PREFABRICATED WOOD DESIGNED AND DETAILED BY THE TRUSS MANUFACTURER FOR THE BUILDING CODE (WIND/SEISMIC/SNOW) AND LOADING SPECIFIED IN THESE DOCUMENTS.

LINTEL / BEAM SCHEDULE
(BEARING AND NON-BEARING WALLS)

MARK	WOOD
L-1	3-1/2" X 11-7/8" BCI VERSA-LAM LVL 1.8E 2650
L-2	1-3/4" X 9-1/2" BCI VERSA-LAM LVL 1.8E 2650

FOOTING SCHEDULE

MARK	SIZE	REINFORCING
F2424	2'-0" X 2'-0" X 12 D."	(3) #4 EW T&B
F26	2'-6" DIA. X 12 D."	
F36	3'-0" DIA. X 12 D."	

PIER SCHEDULE

MARK	SIZE	DESCRIPTION
P1	PC8500	PERMA COLUMN
P2	PC6600	PERMA COLUMN

PIER NOTES:

1. PIERS ARE TO BE AS MANUFACTURED BY PERMA-COLUMN, LLC. SUBMIT ENGINEERING DATA FOR EACH TYPICAL COLUMN.
2. PIER INSTALLATION TO BE IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS AND ENGINEERING DATA.
3. PIERS AND PIER ANCHORING TO BE COORDINATED WITH THE ENGINEERED COLUMN SIZES.

COLUMN SCHEDULE

MARK	SIZE
C1	7" X 7" BCI VERSA-LAM LVL 1.8E 2650
C2	7" X 5-1/4" BCI VERSA-LAM LVL 1.8E 2650
C3	3-1/2" X 5-1/4" BCI VERSA-LAM LVL 1.8E 2650

COLUMN NOTES:

1. BCI VERSA-LAM SIZING REFERENCED WITHIN THE SCHEDULES ABOVE ARE ACTUAL MEMBER SIZES.
2. BCI VERSA-LAM SIZING BASED ON BOISE BCI VERSA-LAM / BOISE CASCADE ENGINEERED WOOD PRODUCTS ENGINEERING DATA, AND HAS BEEN PROVIDED FOR REFERENCE ONLY. THE POST FRAME BUILDING CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE BUILDING STRUCTURAL FRAME AND SIZING OF ALL FRAME MEMBERS.
3. 7" X 7" AND 7" X 5-1/4" NOMINAL LAMINATED COLUMNS SHALL BE PROVIDED WITH CUSTOM CAST IN PLACE CONCRETE COLUMN POST BASE W/ ANCHORS, 6-3/4" X 6-3/4", WITH (4) 10-1/2" H. X 3-1/2 W., 10 GA. STRAPS, PROVIDE EA. W/ (2) 5/8" BOLTS, BLOCK BOTTOM 10" OF COLUMN.

STRUCTURAL DESIGN REQUIREMENTS:

1. FRAMING DESIGN: STRUCTURAL FRAMING DESIGN INFORMATION WITHIN THESE CONTRACT DOCUMENTS HAS BEEN PROVIDED FOR REFERENCE ONLY. IT SHALL BE THE RESPONSIBILITY OF THE POST FRAME BUILDING CONTRACTOR TO RETAIN THE SERVICES OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW JERSEY TO DESIGN THE COMPLETE POST FRAMED BUILDING STRUCTURAL FRAME. THE POST FRAMED BUILDING CONTRACTOR'S PROFESSIONAL ENGINEER SHALL PROVIDE SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS INCLUDING A CERTIFIED TRUSS SUBMITTAL PACKAGE, WITH TRUSS LAYOUTS SHOWING ALL BEARING WALL SUPPORTS AS DEFINED IN THE 2021 NJ INTERNATIONAL BUILDING CODE FOR REVIEW PRIOR TO ANY FABRICATION. ADDITIONAL BEARING WALLS OR POSTS REQUIRED BY DESIGN BUT NOT SHOWN ON THE ORGINAL DESIGN DRAWINGS SHALL BE HIGHLIGHTED. THE POST FRAMED BUILDING CONTRACTOR'S PROFESSIONAL ENGINEER SHALL DESIGN AND PROVIDE PREFABRICATED ANCHORS AS REQUIRED, AS WELL AS TRUSS TO TRUSS AND TRUSS TO FRAME CONNECTIONS IN COMPLIANCE WITH CURRENT BUILDING CODE.
2. STRUCTURAL DESIGN COORDINATION: THE POST FRAMED BUILDING CONTRACTOR'S PROFESSIONAL ENGINEER SHALL PROVIDE ALL STRUCTURAL REACTIONS NECESSARY FOR THE DESIGN OF THE FOUNDATION SYSTEMS.

DATE	No.	REVISION
10/30/26	1	RELEASE FOR BID

LOT NO. 3
BLOCK NO. 4704

LRHSD PROJECT NO. REF21-43-8319, 2-010-24
PROPOSED NEW MAINTENANCE FACILITY

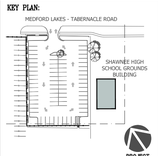
SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING

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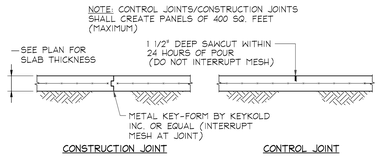
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SCALE:	AS NOTED	DRAWN BY:	DCD
COMM. No.	8319	CHECKED BY:	DCD
DATE:	11/1/2024		

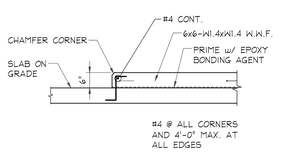
SHEET TITLE:
STRUCTURAL PLANS AND DETAILS AND DETAILS

SHEET No.:
S1.1

NOTE: CONTROL JOINTS/CONSTRUCTION JOINTS SHALL CREATE PANELS OF 400 SQ. FEET (MAXIMUM)

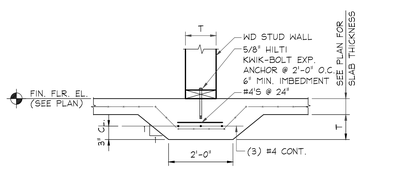


TYPICAL SLAB ON GRADE DETAILS

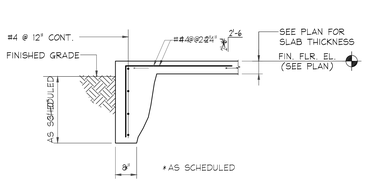


TYPICAL HOUSEKEEPING PAD DETAIL

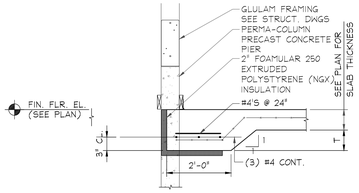
NOTES:
1. SEE PLANS FOR FINISHED FLOOR ELEVATIONS. SEE ARCHITECTURAL DOCUMENTS FOR ALL SLOPES AND VARIANCES FROM ELEVATION SHOWN. DATUM ELEVATION (0.00').
2. +/-___ INDICATES TOP OF SLAB ELEVATION
3. (___) INDICATES TOP OF FOOTING ELEVATION RELATIVE TO DATUM ELEVATION 0.00'
4. FOOTINGS, COLUMNS AND PIERS ARE CENTERED ON THE WALL, UNLESS NOTED OTHERWISE.
5. ALL EXPOSED WOOD SHALL BE ACQ. TREATED TYPICAL
6. "TS" INDICATES THICKENED SLAB. SEE TYPICAL DETAIL THIS SHEET.
7. COORDINATE ALL UNDERGROUND UTILITY REQUIREMENTS WITH CIVIL / MPE DRAWINGS. ALL UTILITIES SHALL BE ABOVE / BELOW FOOTINGS AND NOT LOCATED WITHIN FOOTINGS. NOTIFY ARCHITECT / ENGINEER OF RECORD IF OTHERWISE.



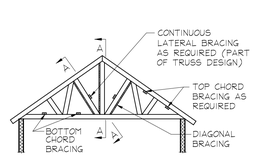
TYPICAL THICKENED SLAB EDGE DETAIL
(SEE ARCH. DWGS. FOR WALL LOCATIONS, TYP. AT EXTERIOR BEARING WALLS)



TYPICAL TURNED DOWN SLAB DETAIL



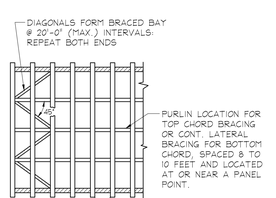
TYPICAL SLAB EDGE DETAIL
(SEE ARCH. DWGS. FOR WALL LOCATIONS, TYP. AT EXTERIOR WALLS)



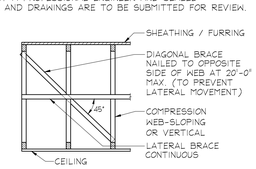
TYPICAL TRUSS ELEVATION

GENERAL NOTES

1) WOOD TRUSSES SHALL BE BRACED AND ERECTED IN ACCORDANCE WITH THE "TRUSS PLATE INSTITUTE" STANDARDS. SEE GENERAL NOTE.
2) BRACING IN THE PLANE OF WEB MEMBERS:
   a. THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING FOR EACH TRUSS WEB MEMBER AS REQUIRED.
   b. LATERAL BRACING SHALL BE RESTRAINED BY DIAGONAL BRACING (MIN. 2" THICK NOMINAL LUMBER). THIS BRACING IS TO BE CONTINUOUS.
   c. A MINIMUM OF TWO ROWS OF DIAGONAL BRACING IS REQUIRED, ONE AT EACH VERTICAL WEB MEMBER CLOSEST TO BEARING LOCATIONS.
3) THE BOTTOM CHORDS SHALL BE BRACED BY CONTINUOUS LATERAL BRACING SPACED AT 8 TO 10 FEET NAILED TO TOP OF THE BOTTOM CHORD. DIAGONALS PLACED AT 45° TO THE LATERAL BRACES SHALL BE LOCATED AT EACH END, IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.
4) TOP CHORD BRACING
   a. IF PLYWOOD DECKING IS APPLIED DIRECTLY TO TOP CHORD, PROPERLY LAPPED AND NAILED TO DEVELOP DIAPHRAGM ACTION, BRACING IS NOT REQUIRED.
   b. IF PURLINS ARE USED, DIAGONAL TOP CHORD BRACING IS REQUIRED AT EACH END. IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.
5) WOOD ROOF TRUSSES ARE TO BE DESIGNED FOR THE WOOD FABRICATOR BY A PROFESSIONAL ENGINEER AND SEALED CALCULATIONS AND DRAWINGS ARE TO BE SUBMITTED FOR REVIEW.



TOP/BOTTOM CHORD BRACING



SECTION A-A

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REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION - TABLE 1705.3

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	–	X	ACI 318: Ch. 20, 25.2, 25.3, 26.5-26.5.3	1908.4
2. REINFORCING BAR WELDING:				–
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706;	–	X	AWS D1.4	
b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND		X	ACI 318: 26.5.4	
c. INSPECT ALL OTHER WELDS.	X			
3. INSPECTION OF ANCHORS CAST IN CONCRETE.	–	X	ACI 318:17.8.2	–
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.				–
a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	X		ACI 318: 17.8.2.4	
b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.		X	ACI 318: 17.8.2	
5. VERIFYING USE OF REQUIRED DESIGN MIX	–	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	–	ASTM C172 ASTM C31 ACI 318: 26.4.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	–	ACI 318: 26.4.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	–	X	ACI: 318: 26.4.7-26.4.9	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR:				–
a. APPLICATION OF PRESTRESSING FORCES; AND GROUTING OF BONDED PRESTRESSING TENDONS	X	–	ACI 318: 26.9.2.1	
b. GROUTING OF BONDED PRESTRESSING TENDONS.	X	–	ACI 318: 26.9.2.3	
10. INSPECT ERECTION OF PRECAST	–	X	ACI 318: Ch. 26.8	–
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	–	X	ACI 318: 26.10.2	–
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	–	X	ACI 318: 26.10.1	–

REQUIRED VERIFICATION AND INSPECTION OF SOILS - TABLE 1705.6

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	–	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	–	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	–	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	–
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	–	X

SIMPSON STRONG-TIE TIEDOWN CONNECTORS FOR ROOF TRUSSES

NOTE: G.C. TO PROVIDE A MINIMUM OF (2) WALL STUDS FOR UPLIFT LOADS < 3000#, (3) WALL STUDS FOR UPLIFT LOADS BETWEEN 3000# & 7000# & (4) WALL STUDS FOR UPLIFT LOAD BETWEEN 7000# AND 11000# UNLESS GREATER/LARGER POST/STUDS ARE SHOWN ON DOCUMENTS.

ALLOWABLE UPLIFT LOAD - LBS.	NO. OF PLIES	NON WRAP-OVER CONNECTOR	WRAP-OVER CONNECTOR	VERTICAL HEIGHT OF TRUSS REQ'D. BY CONNECTOR ABOVE PLATE	POST/WALL STUD(S) FOUNDATION TIE-DOWN	MULTI-STORY FLOOR-FLOOR TIES
150	ANY	(3) 10d NAILS	-	NONE	NONE	NONE
200	ANY	(4) 10d NAILS	-	NONE	NONE	NONE
535	1	H2.5A	-	3"	NONE	NONE
1,070	ANY	(2) H2.5A DIAGONALLY OPPOSITE	-	3"	(1) LTT20B, (1) 5/8"Ø BOLT, 12" EMBED.	CS16-R OR FSC ABOVE & BELOW WITH 3/8"Ø ALL-THREAD
1,120	1	-	H15	3"	(1) LTT20B, (1) 5/8"Ø BOLT, 12" EMBED.	CS16-R OR FSC ABOVE & BELOW WITH 3/8"Ø ALL-THREAD
1,120	2	-	H15-2	3"	(1) LTT20B, (1) 5/8"Ø BOLT, 12" EMBED.	CS16-R OR FSC ABOVE & BELOW WITH 3/8"Ø ALL-THREAD
1,245	ANY	HTS30C	-	16"	(1) LTT20B, (1) 5/8"Ø BOLT, 12" EMBED.	CS16-R OR FSC ABOVE & BELOW WITH 3/8"Ø ALL-THREAD
1,750	2	LGT2	-	3"	(1) LTT20B, (1) 5/8"Ø BOLT, 12" EMBED.	CS14 OR HTT16 ABOVE & BELOW WITH 5/8"Ø ALL-THREAD
2,140	AT LEAST 2	(4) H2.5A	-	3"	(1) HTT16, (1) 5/8"Ø BOLT, 12" EMBED.	CMSTC16 OR HTT16 ABOVE & BELOW WITH 5/8"Ø ALL-THREAD
3,170	3	LGT3-SDS2.5	-	3"	(1) HTT16, (1) 5/8"Ø BOLT, 12" EMBED.	CMSTC16 OR HTT16 ABOVE & BELOW WITH 5/8"Ø ALL-THREAD
3,330	AT LEAST 2	-	MGT, (1) HTT16, (1) 5/8"Ø BOLT	3"	(1) HTT16, (1) 5/8"Ø BOLT, 12" EMBED.	CMSTC16 OR HTT16 ABOVE & BELOW WITH 5/8"Ø ALL-THREAD
5,250	ANY	(1) HTT22 ON TRUSS & (1) HTT22 ON POST WITH (1) 5/8"Ø BOLT	-	24"	(1) HTT22, (1) 5/8"Ø BOLT, 12" EMBED.	CMST14 OR HTT22 ABOVE & BELOW WITH 5/8"Ø ALL-THREAD
6,485	2	-	HGT-2, (2) HTT16, (2) 5/8"Ø BOLTS	3"	(1) HTT16 & (1) 5/8"Ø BOLT EA. SIDE , 12" EMBED.	CMST14 OR HTT16 EA. SIDE ABOVE & BELOW WITH 5/8"Ø ALL-THREAD
9,035	3	-	HGT-3, (2) HTT22, (2) 5/8"Ø BOLTS	3"	(1) HTT22 & (1) 5/8"Ø BOLT EA. SIDE , 12" EMBED.	CMST12 OR HTT22 EA. SIDE ABOVE & BELOW WITH 5/8"Ø ALL-THREAD
10,500	ANY	(1) HTT22 ON EA. SIDE OF TRUSS & (1) HTT22 ON EA. SIDE OF POST WITH (1) 5/8"Ø BOLT EA. SIDE	-	24"	(1) HTT22 & (1) 5/8"Ø BOLT EA. SIDE , 12" EMBED.	HTT22 EA. SIDE ABOVE & BELOW WITH 5/8"Ø ALL-THREAD

NOTE: CONTRACTOR TO SELECT TIE-DOWN CONNECTOR FOR ALL ROOF TRUSSES WITH STATED UPLIFT CAPACITY THAT MEETS OR EXCEEDS ACTUAL NET UPLIFT VALUES STATED ON TRUSS PIECE DRAWINGS.

SPECIAL INSPECTION REQUIREMENTS - IBC/NJ 2021 EDITION

1. THE QUALIFIED AGENCY AND PERSONNEL RETAINED FOR THESE SPECIAL INSPECTION SERVICES SHALL BE APPROVED BY THE OWNER, THE ARCHITECT AND THE ENGINEER OF RECORD PRIOR TO START OF CONSTRUCTION. AN OUTLINE OF THE SCOPE OF SERVICES TO BE PERFORMED BY THE INSPECTING AGENCY IS TO BE SUBMITTED PRIOR TO START OF CONSTRUCTION.

2. IN ACCORDANCE WITH SECTION 1705 OF THE INTERNATIONAL BUILDING CODE 2021 EDITION, WITH AMENDMENTS PER NJ EDITION 2015, PERIODIC AND/OR CONTINUOUS INSPECTIONS OF THE CONSTRUCTION PROGRESS SHALL BE MADE BY AN INDEPENDENT APPROVED AGENCY IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS;

CONCRETE CONSTRUCTION — SECTION 1705.3, TABLE 1705.3
SOILS — SECTION 1705.6, TABLE 1705.6

SNOW DESIGN LOAD SCHEDULE
INTERNATIONAL BUILDING CODE (2021)

ITEM	SYMBOL	VALUE	REFERENCE
GROUND SNOW LOAD	P_g	20	FIGURE 1608.2
SNOW EXPOSURE FACTOR	C_e	1.0	TABLE 7-2
SNOW LOAD IMPORTANCE FACTOR	I_s	1.0	TABLE 1.5-2
THERMAL FACTOR	C_t	1.0	TABLE 7-3
FLAT-ROOF SNOW LOAD	P_f	20	SECTION 7.3

LATERAL LOAD DESIGN SCHEDULE
INTERNATIONAL BUILDING CODE (2021)

WIND LOAD

ITEM	SYMBOL	VALUE	REFERENCE
BASIC WIND SPEED (3 SEC. GUST)	V	131	FIGURE 1609
WIND LOAD IMPORTANCE FACTOR	–	1.00	TABLE 1604.5
WIND EXPOSURE CATEGORY	-	B	SECTION 1609.4.3

SEISMIC LOAD

ITEM	SYMBOL	VALUE	REFERENCE
IMPORTANCE FACTOR	I_E	1.25	TABLE 1604.5
SHORT PERIOD SPECTRAL ACCELERATION	S_{DS}	0.288	SECTION 1613.3.4
(1) SECOND PERIOD SPECTRAL ACCELERATION	S_{D1}	0.114	SECTION 1613.3.4
OCCUPANCY CATEGORY	-	III	SECTION 1604.5
SEISMIC DESIGN CATEGORY	-	B	TABLE 1613
SITE CLASSIFICATION	-	D	TABLE 20.3-1
BASIC SEISMIC-RESISTING SYSTEM	-	LIGHT FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS	TABLE 12.2-1
RESPONSE MODIFICATION FACTOR	R	6.5	TABLE 12.2-1
DEFLECTION AMPLIFICATION FACTOR	C_d	4	TABLE 12.2-1
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE PROCEDURE		SECTION 12.0

DESIGN LOAD SCHEDULE
(ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)

COMPONENT \ AREA	SLAB ON GRADE	EQUIP PLATFORM	ROOF			
CONCRETE SLAB	100					
FRAMING		10	5			
CEILING		5	2			
COLLATERAL		10	3			
ROOFING & INSULATION			10			
TOTAL DEAD LOAD	50	25	25			
TOTAL LIVE LOAD	100	100	30			
TOTAL LOAD	150	125	55			

STRUCTURAL DESIGN REQUIREMENTS:

1. FRAMING DESIGN: STRUCTURAL FRAMING DESIGN INFORMATION WITHIN THESE CONTRACT DOCUMENTS HAS BEEN PROVIDED FOR REFERENCE ONLY. IT SHALL BE THE RESPONSIBILITY OF THE POST FRAME BUILDING CONTRACTOR TO RETAIN THE SERVICES OF A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW JERSEY TO DESIGN THE COMPLETE POST FRAMED BUILDING STRUCTURAL FRAME. THE POST FRAMED BUILDING CONTRACTOR'S PROFESSIONAL ENGINEER SHALL PROVIDE SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS INCLUDING A CERTIFIED TRUSS SUBMITTAL PACKAGE, WITH TRUSS LAYOUTS SHOWING ALL BEARING WALL SUPPORTS AS DEFINED IN THE 2021 NJ INTERNATIONAL BUILDING CODE FOR REVIEW PRIOR TO ANY FABRICATION. ADDITIONAL BEARING WALLS OR POSTS REQUIRED BY DESIGN BUT NOT SHOWN ON THE ORIGINAL DESIGN DRAWINGS SHALL BE HIGHLIGHTED. THE POST FRAMED BUILDING CONTRACTOR'S PROFESSIONAL ENGINEER SHALL DESIGN AND PROVIDE PREFABRICATED ANCHORS AS REQUIRED, AS WELL AS TRUSS TO TRUSS AND TRUSS TO FRAME CONNECTIONS IN COMPLIANCE WITH CURRENT BUILDING CODE.

2. STRUCTURAL DESIGN COORDINATION: THE POST FRAMED BUILDING CONTRACTOR'S PROFESSIONAL ENGINEER SHALL PROVIDE ALL STRUCTURAL REACTIONS NECESSARY FOR THE DESIGN OF THE FOUNDATION SYSTEMS.

GENERAL CONSTRUCTION

1. NOTES, TYPICAL DETAILS, AND SCHEDULES APPLY TO ALL STRUCTURAL WORK UNLESS NOTED OTHERWISE. TYPICAL DETAILS ARE TO BE USED FOR ALL CONDITIONS WHERE THE DETAIL IS APPLICABLE, WHETHER OR NOT NOTED ON PLAN. TYPICAL DETAILS MAY BE SLIGHTLY ALTERED IF REQUIRED DUE TO PROJECT CONDITIONS, ONLY WHEN SUBMITTED AND THE ENGINEER'S APPROVAL IS OBTAINED PRIOR TO PERFORMING THE WORK.
2. ALL DIMENSIONS AND ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS, WITH THE EXCEPTION OF STRUCTURAL MEMBER SIZES, ARE GENERATED BY OTHER DISCIPLINES. ANY DIMENSIONS OR ELEVATIONS OMITTED OR NOT SHOWN ON THE STRUCTURAL DRAWINGS SHOULD BE OBTAINED FROM THE DRAWINGS OF THE OTHER DISCIPLINES. STRUCTURAL DRAWINGS ARE NOT "STAND-ALONE" DOCUMENTS AND SHOULD BE USED IN CONJUNCTION WITH, AND COORDINATED WITH THE SPECIFICATIONS, ARCHITECTURAL DRAWINGS AND ALL OTHER DISCIPLINE'S DRAWINGS. IF THERE IS A DISCREPANCY BETWEEN DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER AND ARCHITECT PRIOR TO PERFORMING THE WORK.
3. IF DIFFERENCES OCCUR WITHIN OR BETWEEN DRAWINGS AND SPECIFICATIONS REGARDING MATERIALS, STRENGTHS OR QUANTITIES, THE BETTER MATERIAL, HIGHER STRENGTH, AND GREATER QUANTITY INDICATED, SPECIFIED OR NOTED SHALL BE PROVIDED.
4. DO NOT SCALE DRAWINGS TO OBTAIN DIMENSIONAL INFORMATION.
5. THESE DRAWINGS DO NOT DEFINE SCOPE OF CONTRACTOR OR SUBCONTRACTOR CONTRACTS.
6. AT ALL TIMES, THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOBSITE INCLUDING MEANS AND METHODS OF CONSTRUCTION AND SAFETY OF PERSONS AND PROPERTY. THE ENGINEER'S PRESENCE OR REVIEW OF WORK AT THE JOBSITE IS FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT ONLY AND IS NOT EVER TO BE CONSTRUED AS A REVIEW OF MEANS AND METHODS OF CONSTRUCTION AND SAFETY METHODS.
7. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING ALLOWABLE CONSTRUCTION LOADS AND FOR PROTECTING THE COMPLETED OR INCOMPLETED STRUCTURAL FRAMING FROM DAMAGE DUE TO TEMPORARY CONSTRUCTION LOADINGS.
8. COSTS OF INVESTIGATION AND/OR REDESIGN DUE TO CONTRACTOR ERRORS WILL BE AT THE CONTRACTOR'S EXPENSE.
9. ANY APPROVED CONTRACTOR REQUESTED CHANGES TO THESE DRAWINGS WILL BE DONE AT NO COST TO THE OWNER. APPROVAL OF CONTRACTOR REQUESTED CHANGES IN NO WAY STATES OR IMPLIES APPROVAL OF A CHANGE IN SCOPE OR CHANGE IN CONTRACT COST.
10. UNLESS EXPLICITLY NOTED AS "ISSUED FOR BID", THESE DRAWINGS ARE NOT SUITABLE FOR OBTAINING BIDS FROM GENERAL OR SUBCONTRACTORS. BIDDING OF DRAWINGS PRIOR TO DESIGN COMPLETION AND "ISSUED FOR BID" IS DONE AT THE SOLE RISK OF THE BIDDING CONTRACTOR. ADDITIONS OR CORRECTIONS TO DRAWINGS THAT ARE BID PRIOR TO DESIGN COMPLETION AND "ISSUED FOR BID" WILL NOT BE CONSIDERED AS DESIGN ERRORS OR OMISSIONS. STRUCTURAL DESIGN, BY NATURE, CANNOT BE COMPLETE PRIOR TO COMPLETION OF ARCHITECTURAL AND MECHANICAL DRAWINGS.
11. ALL REFERENCES TO WATER/DAMPROOFING, FIREPROOFING AND UTILITIES ON THE STRUCTURAL DRAWINGS ARE FOR REFERENCE ONLY. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ALL WATER/DAMPROOFING, FIREPROOFING AND UTILITY DETAILS AND REQUIREMENTS.
12. IF THE EXISTING FIELD CONDITIONS DO NOT PERMIT THE INSTALLATION OF THE WORK IN ACCORDANCE WITH THE DETAILS SHOWN, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY. THE CONTRACTOR MUST PROVIDE A SKETCH OF THE CONDITION WITH HIS PROPOSED MODIFICATION OF THE DETAILS GIVEN ON THE CONTRACT DOCUMENTS. THIS SKETCH MUST BE SUBMITTED TO AND APPROVAL MUST BE GRANTED BY THE ENGINEER PRIOR TO PERFORMING THE WORK.
13. SUBMIT SHOP DRAWINGS SUCH THAT BY THE TIME THEY ARE RECEIVED BY THE DESIGN COLLABORATIVE THERE WILL BE AT LEAST 14 DAYS BEFORE REVIEWED SUBMITTALS WILL BE NEEDED. ANY REVIEW THAT IS REQUIRED MORE EXPEDIENTLY WILL BE AT THE CONTRACTOR'S EXPENSE. SHOP DRAWINGS SHALL BEAR THE CONTRACTOR'S STAMP OF APPROVAL CERTIFYING THAT HE HAS VERIFIED ALL FIELD MEASUREMENTS, CONSTRUCTION CRITERIA, MATERIALS AND SIMILAR DATA AND HAS CHECKED EACH DRAWING FOR COMPLETENESS, COORDINATION AND COMPLIANCE WITH THE CONTRACT DOCUMENTS. IF REVIEW OF AN INCOMPLETE SHOP DRAWING IS REQUIRED, THAT SHOP DRAWING SHALL BE CLEARLY MARKED AS INCOMPLETE. THE AREA THAT NEEDS TO BE REVIEWED SHALL BE CLEARLY NOTED WITH AN EXPLANATION FOR THE REASON FOR PARTIAL APPROVAL.
14. SIZE AND/OR LOCATION OF OPENINGS, SLEEVES, CONCRETE HOUSEKEEPING PADS, INSERTS, DEPRESSIONS, ETC. SHOWN ON THE STRUCTURAL DOCUMENTS ARE FOR THE CONTRACTOR'S CONVENIENCE ONLY. THE CONTRACTOR IS SOLELY RESPONSIBLE TO COORDINATE ALL CONTRACT DOCUMENTS TO DETERMINE THE SIZE AND/OR LOCATION OF OPENINGS, SLEEVES, CONCRETE HOUSEKEEPING PADS, INSERTS, DEPRESSIONS, ETC.

FOUNDATIONS

1. PERFORM ALL SITE PREPARATION AND EXCAVATION WORK IN STRICT ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING REPORT AS PREPARED BY CRAIG TESTING LABORATORIES (*******, 2021).
2. EXCAVATE THE BUILDING SITE TO THE DEPTH AND EXTENT INDICATED IN THE SOILS REPORT. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING.
3. BOTTOM OF FOOTINGS SHALL BEAR ON CONTROLLED COMPACTED FILL CAPABLE OF SAFELY SUPPORTING 3000 PSF.
4. SUBGRADE OF ALL FOOTINGS MUST BE INSPECTED UNDER THE SUPERVISION OF AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE PLACING ANY CONCRETE. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.
5. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 4'-0" FEET BELOW EXTERIOR FINISH GRADE. ALL FOOTING ELEVATIONS SHOWN ON PLAN ARE THE BEST APPROXIMATIONS SHOWN BASED ON AVAILABLE DATA. GENERAL CONTRACTOR MAY ALTER FOOTING ELEVATIONS FOR REASONS INCLUDING, BUT NOT LIMITED TO REVISED GEOTECHNICAL OR CIVIL INFORMATION, UNFORESEEN CONDITIONS, ACTUAL INVERT ELEVATIONS, CONSTRUCTABILITY, ETC. CONTRACTOR SHALL NOTIFY ARCHITECT AND OBTAIN WRITTEN APPROVAL PRIOR TO ANY MODIFICATIONS.
6. DO NOT BACKFILL ANY BASEMENT WALLS WITH AN UNBALANCED HEIGHT OF SOIL GREATER THAN THREE FEET UNTIL ELEVATED FLOOR IS IN-PLACE AND THE WALL HAS REACHED ITS DESIGN STRENGTH OR THE WALLS ARE ADEQUATELY BRACED.
7. EXPOSED CONCRETE/CMU WALLS SHALL HAVE CONTROL JOINTS AT 30 FEET MAXIMUM ON CENTER UNLESS NOTED OTHERWISE. WALLS WITH INTEGRAL COLUMN PIERS OR PILASTERS SHALL BE POURED MONOLITHICALLY AND SHALL HAVE A FORMED CONTROL JOINT ON ONE SIDE OF EACH PIER ON THE EXPOSED FACE OF THE WALL. JOINTS SHALL BE FILLED WITH AN APPROVED SEALANT.

CONCRETE

1. REINFORCING STEEL SHALL HAVE A MINIMUM CLEAR COVER AS FOLLOWS, UNLESS NOTED OTHERWISE ON DRAWINGS:
   CONCRETE POURED AGAINST EARTH — 3"

   CONCRETE EXPOSED TO EARTH OR WEATHER:
   #5 OR SMALLER — 1 1/2"
   #6 OR LARGER — 2"

   CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
   SLABS, WALLS, JOISTS:
   #14 OR #18 BARS — 1 1/2"
   #11 OR SMALLER — 3/4"

   CLEAR COVER SHALL BE CLEARLY SHOWN ON ALL REINFORCING BAR DETAIL DRAWINGS.
2. ALL CONCRETE SHALL BE READY-MIX AND HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF:

   SPREAD FOOTINGS/WALL FOOTINGS/FOUNDATION WALL
   BASEMENT WALLS/RETAINING WALLS — 3,000 PSI
   PIERS-MATCH WALL STRENGTH — (MINIMUM OF 3,000 PSI)
   SLAB-ON-GRADE — 4,000 PSI

   HAVE A MINIMUM OF 500 LBS. OF CEMENT PER CUBIC YARD. SLUMP (AT POINT OF CONCRETE PLACEMENT) SHALL BE 3 INCH MINIMUM AND 6 INCH MAXIMUM.

   CONCRETE EXPOSED TO WEATHER SHALL HAVE 5 PERCENT AIR ENTRAINMENT. CONCRETE NOT EXPOSED TO WEATHER SHALL NOT CONTAIN
   AN AIR-ENTRAINING AGENT. SUBMIT MIX DESIGNS FOR REVIEW.

   CONCRETE TO BE GIVEN A HARD-TROWELED FINISH SHALL NOT CONTAIN AN AIR-ENTRAINING AGENT. TOTAL AIR CONTENT FOR THIS CONCRETE SHOULD NOT EXCEED 3 PERCENT (AT POINT OF CONCRETE PLACEMENT). ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE LATEST ACI BUILDING CODE (ACI 318), THE ACI DETAILING MANUAL (ACI 315), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301).
3. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A615 GRADE 60. WWF SHALL COMPLY WITH ASTM A185.
4. LAP ALL REINFORCING BARS 62 DIAMETERS. LAP ALL WWF A MINIMUM OF SIX INCHES.
5. ALL INSERTS AND SLEEVES SHALL BE CAST-IN-PLACE. THE CONTRACTOR SHALL VERIFY THE DIMENSIONS AND LOCATIONS OF ALL OPENINGS, PIPE SLEEVES, ETC. AS REQUIRED BY ALL TRADES BEFORE THE CONCRETE IS POURED. THE CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS, AS WELL AS THE STRUCTURAL DRAWINGS FOR THE LOCATION, NUMBER, AND SIZE OF ALL OPENINGS, SLEEVES, ETC. HOWEVER, OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE INSTALLED ONLY AFTER APPROVAL BY THE STRUCTURAL ENGINEER IS OBTAINED. DRAWINGS SHALL BE SUBMITTED FOR REVIEW SHOWING LOCATIONS AND DIMENSIONS OF ALL OPENINGS, SLEEVES, ETC. IN CAST-IN-PLACE CONCRETE SLABS, BEAMS, WALLS, COLUMNS, AND FOUNDATIONS. THESE DRAWINGS SHALL BE COORDINATED BY THE CONTRACTOR. OPENINGS AND SLEEVES THROUGH CAST-IN-PLACE CONCRETE FRAMING IS PROHIBITED EXCEPT WHERE THOSE SLEEVES AND OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS OR WHERE THEY ARE SHOWN ON THE APPROVED SLEEVE AND OPENING DRAWINGS THAT HAVE BEEN SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. SAW-CUTTING, CORING, OR DRILLING OF SLEEVES OR OPENING THROUGH PREVIOUSLY CAST CONCRETE IS NOT PERMITTED EXCEPT WHERE SPECIFICALLY REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER.
6. LOCATION OF CONSTRUCTION JOINTS IN THE STRUCTURAL SLAB SHALL BE SUBMITTED FOR APPROVAL BY THE STRUCTURAL ENGINEER. CONSTRUCTION JOINTS IN STRUCTURAL SLABS AND GRADE BEAMS SHALL BE AT MID-SPAN AND KEY JOINTED WITH REINFORCING CONTINUOUS ACROSS JOINT. CONSTRUCTION JOINTS IN SLABS ON METAL DECK SHALL OCCUR MIDWAY BETWEEN BEAMS AT END THIRD OF GIRDER SPAN.
7. THE CONTRACTOR SHALL SUPPLY FLOOR LEVELING MATERIAL AND OTHER CORRECTIVE MEASURES IN AREAS WHERE FLOOR FINISH PROVISIONS EXCEED THE FLATNESS AND LEVELNESS REQUIREMENTS.

TIMBER

1. ALL STRUCTURAL TIMBER SHALL BE SOUTHERN YELLOW PINE, MINIMUM, STRESS GRADE LUMBER OR APPROVED EQUAL. THE MINIMUM ALLOWABLE PROPERTIES ARE AS FOLLOWS: Fb = 1,250 PSI; Fv = 175 PSI; E = 1,100,000 PSI
2. ALL STRUCTURAL TIMBER TO BE STAMPED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION'S "CONSTRUCTION MANUAL".
3. ALL PARALLAM BEAMS TO BE AS ENGINEERED AND MANUFACTURED BY TRUSS JOIST MACMILLAN OR APPROVED EQUAL. THE MINIMUM ALLOWABLE PROPERTIES ARE AS FOLLOWS: Fb = 2,900 PSI; Fv = 290 PSI; E = 2,000,000 PSI
4. ALL TIMBER AND TIMBER CONSTRUCTION SHALL COMPLY WITH 2009 UBC NJ EDITION CHAPTER 23 AND FOLLOWING CODES AND SPECIFICATIONS REFERENCED WITHIN THE 2009 IBC NJ EDITION CHAPTER 23:
   - 4.1. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION: TIMBER CONSTRUCTION MANUAL
   - 4.2. NATIONAL FOREST PRODUCTS ASSOCIATION: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
   - 4.3. AMERICAN PLYWOOD ASSOCIATION PLYWOOD DESIGN SPECIFICATION.
   - 4.4. AMERICAN WOOD-PRESERVERS ASSOCIATION STANDARDS
   - 4.5. NATIONAL LUMBER MANUFACTURERS ASSOCIATION: NATIONAL DESIGN SPECIFICATION FOR STRESS GRADE LUMBER AND ITS FASTENINGS.
5. ALL TIMBER CONNECTIONS SHALL BE MADE USING PREFABRICATED CONNECTORS, TOE-NAILING IS NOT PERMITTED. SUBMIT MANUFACTURER'S DATA FOR REVIEW. FASTENERS SHALL BE AS MANUFACTURED BY HECKMAN, KANT DAG, SIMPSON OR APPROVED EQUAL.
6. PROVIDE PRESSURE TREATED LUMBER WHERE LUMBER IS IN CONTACT WITH CONCRETE, OR IS LOCATED OUTSIDE OF THE BUILDING.
7. ALL WOOD FRAMING CONNECTIONS SHALL BE MADE USING PREFABRICATED CONNECTIONS. PROVIDE STAINLESS STEEL FASTENERS, ANCHORS AND CONNECTORS WITH TREATED WOOD. TOE NAILING IS NOT PERMITTED UNLESS NOTED OTHERWISE IN THE GOVERNING BUILDING CODE. SUBMIT MANUFACTURER'S DATA FOR REVIEW. FASTENERS SHALL BE AS MANUFACTURED BY SIMSPSON STRONG TIE OR APPROVED EQUAL.
8. ALL WOOD TRUSS MEMBERS SHALL BE FABRICATED FROM KILN DRIED SOUTHERN PINE STRESS GRADE LUMBER OR EQUAL.
9. DESIGN, FABRICATION AND INSTALLATION OF WOOD TRUSSES AND SHEET METAL CONNECTORS SHALL BE IN ACCORDANCE WITH THE FOLLOWING TRUSS PLATE INSTITUTE STANDARDS:
   - A. NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION - ANSI/TPI-1, LATEST EDITION
   - B. RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES, DSB-89
   - C. GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES, BCSI, LATEST EDITION
10. WOOD ROOF TRUSSES ARE TO BE DESIGNED FOR THE WOOD FABRICATOR BY A PROFESSIONAL ENGINEER. SEALED CALCULATIONS ALONG WITH TRUSS SUBMITTAL PACKAGE INCLUDING A TRUSS LAYOUT SHOWING ALL BEARING WALL SUPPORTS AS DEFINED IN THE INTERNATIONAL BUILDING CODE ARE TO BE SUBMITTED FOR REVIEW PRIOR TO ANY FABRICATION. ADDITIONAL BEARING WALLS OR POSTS REQUIRED BY DESIGN BUT NOT SHOWN ON THE ORIGINAL DESIGN DRAWINGS SHALL BE HIGHLIGHTED. TRUSS FABRICATOR SHALL DESIGN AND PROVIDE PREFABRICATED ANCHORS AS REQUIRED OR TRUSS TO TRUSS CONNECTIONS. THE TRUSS MANUFACTURER MUST INCORPORATE IN THE TRUSS DESIGN ADDITIONAL PLYS, SIMPSON TRUSS BEARING ENHANCERS OR OTHER MEASURES AS REQUIRED TO PREVENT SILL PLATE BEARING TYPE FAILURE FOR THE NOTED PLATE SPECIES
11. LAMINATED WOOD FRAMING SHALL BE DESIGNED BY LAMINATE WOOD BEAM MANUFACTURER. SUBMIT SEALED CALCULATIONS FOR REVIEW.
12. HEADERS AT NON-BEARING CONDITIONS SHALL BE AS FOLLOWS:
    OPENING SIZE HEADER

    UP TO 4'0" — (2) 2" X 6"
    4'1" TO 5'0" — (2) 2" X 8"
    5'1" TO 8'0" — (2) 2" X 10"
13. FOR OPENING HEADERS IN NON-BEARING INTERIOR PARTIONS, PROVIDE ONE JACK STUD AND ON KING STUD. FOR OPENING HEADERS IN LOAD-BEARING INTERIOR PARTIONS, PROVIDE TWO JACK STUDS AND ONE KING STUD, OR A POST AS SHOWN ON DRAWINGS.
14. NAIL PILES OF BUILT-UP HEADERS, BEAMS AND STUGGPOSTS TOGETHER WITH TWO ROWS OF 100 NAILS AT 10" SPACING, UNLESS NOTED OTHERWISE ON DOCUMENTS.
15. PROVIDE MINIMUM CONTINUOUS SOLID BLOCKING OR CROSS-BRIDGING LINES AT 8'-4" OC MAXIMUM BRACING FOR ALL ROOF TRUSSES. PROVIDE ADDITIONAL XBRACING AS REQUIRED BY FABRICATOR. PROVIDE A MINIMUM OF ONE LINE OF BLOCKING CROSS BRACING FOR ALL SPANS.
16. PROVIDE STRUCTURAL PLYWOOD SHEATHING OR APPROVED EQUAL AT ALL SIDES OF CORNERS FOR WIND BRACING. CONNECTIONS OF PLYWOOD SHALL COMPLY WITH ALL NAILING REQUIREMENTS FOR PLYWOOD SHEATH WALLS. NO MORE THAN 50% OF ALL SHEATHING JOINTS MAY COINCIDE WITH A CONNECTION LINE BETWEEN FLOORS.
17. PROVIDE AN ADDITIONAL JOIST OR TRUSS UNDER ALL PARTITIONS PARALLEL TO JOISTS.
18. SHEATHING FOR FLOORS SHALL BE 3/4" APA RATED STURD-I-FLOOR 20 O/C EXPOSURE 1 T&G, OR SIMILARLY RATED ORIENTED STRAND BOARD (OSB). ALL JOINTS IN SHEATHING SHALL BE STAGGERED. ALL EDGES IN FLOOR SHEATHING SHALL BE TONGUE & GROOVE. NAILING SHALL COMPLY WITH APA REQUIREMENTS FOR PLYWOOD FLOOR DIAPHRAGMS, UNLESS NOTED OTHERWISE ON DRAWINGS.

STRUCTURAL SPECIAL INSPECTIONS

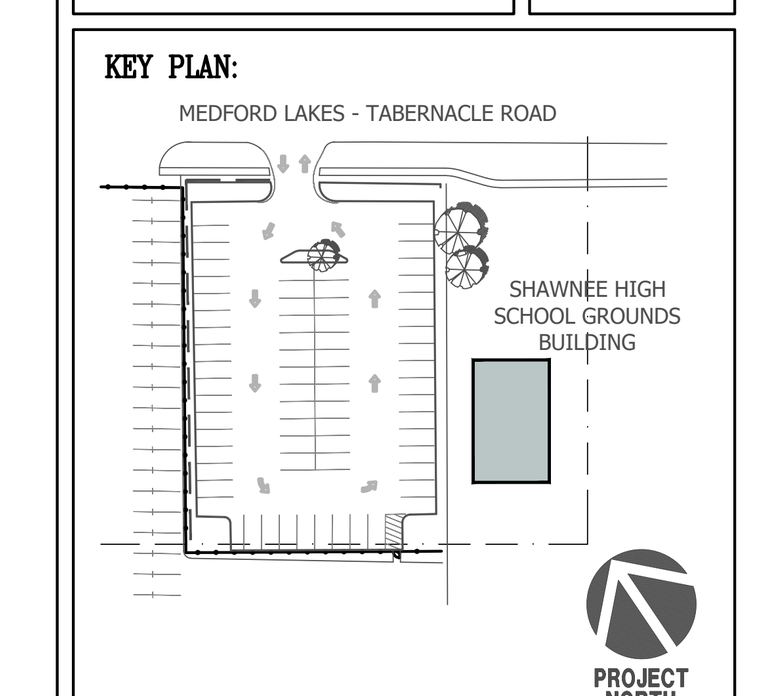
1. THE QUALIFIED AGENCY RETAINED BY THE OWNER FOR THESE SPECIAL INSPECTION SERVICES SHALL BE APPROVED BY THE OWNER, THE ARCHITECT, AND THE ENGINEER OF RECORD PRIOR TO START OF CONSTRUCTION. AN OUTLINE OF THE SCOPE OF SERVICES TO BE PERFORMED BY THE INSPECTING AGENCY IS TO BE SUBMITTED PRIOR TO THE START OF CONSTRUCTION.
2. IN ACCORDANCE WITH SECTION 1704 OF THE INTERNATIONAL BUILDING CODE, AND ALL APPLICABLE STATE AND LOCAL REQUIREMENTS, AN INDEPENDENT APPROVED AGENCY SHALL MAKE PERIODIC AND/OR CONTINUOUS INSPECTIONS OF THE CONSTRUCTION PROGRESS IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:
   STEEL CONCRETE — SECTION 1704.3, TABLE 1704.3
   CONCRETE CONSTRUCTION — SECTION 1704.4, TABLE 1704.4
   SOILS — SECTION 1704.7, TABLE 1704.7

DATE	No.	REVISION
10/30/26	1	RELEASE FOR BID

BLOCK NO. 4704 — LOT NO. 3
LRHSD PROJECT NO. REF21-43-8319, 2-010-24
PROPOSED NEW MAINTENANCE FACILITY
SHAWNEE HIGH SCHOOL MAINTENANCE BUILDING
580 TABERNACLE ROAD, MEDFORD, NEW JERSEY 08055
MEDFORD TOWNSHIP — BURLINGTON COUNTY

Linda M. Viventi, AIA, Secretary/Treasurer
New Jersey Architect LIC.# 21AI01090200; P.P. LIC.# 33LI00653000

Drew C. Dingler, AIA, President
New Jersey LIC.# 21AI01088800

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SCALE:	AS NOTED	DRAWN BY: DCD
COMM. No.	8319	CHECKED BY: DCD
DATE:	11/1/2024	

SHEET TITLE: STRUCTURAL NOTES AND SCHEDULES

SHEET No.: S1.2

FILE G:\PROJECTS\8319\ARCHITECTURAL\A1-A8 SERIES\S1\8319 S1 STRUCTURAL PLANS.DWG DATE 11/5/2026 12:20 PM USER drew dingler